APPENDIX G Preliminary Drainage Analysis for Proposed Heacock Logistics Parking Lot (April 2022)

Heacock Logistics Parking Lot Initial Study/Mitigated Negative Declaration September 27, 2022

PRELIMINARY DRAINAGE ANALYSIS FOR PROPOSED HEACOCK LOGISTICS

PARKING LOT

LST21-0041

CITY OF MORENO VALLEY RIVERSIDE COUNTY, CALIFORNIA

Prepared for:

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APRIL 2022

This report has been prepared by or under the direction of the following registered civil engineer who attests to the technical information contained herein.

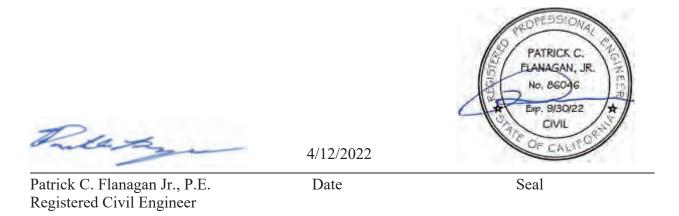


TABLE OF CONTENTS

I.	PURPOSE AND SCOPE	1
II.	PROJECT DESCRIPTION	1
III.	DRAINAGE AREA OVERVIEW	1
IV.	HYDROLOGY	2
V.	STUDY FINDINGS	3
VI.	CONCLUSION	3
VII.	REFERENCES	4

FIGURES

FIGURE 1:REGIONAL VICINITY MAPFIGURE 2:LOCAL VICINITY MAPFIGURE 3:FEMA FLOODPLAIN MAP

APPENDICES

- APPENDIX A: ON-SITE HYDROLOGY BASED ON EXISTING CONDITION (RATIONAL METHOD)
- APPENDIX A.1: 100-YEAR HYDROLOGY CALCULATIONS (EXISTING)
- APPENDIX B: ON-SITE HYDROLOGY BASED ON PROPOSED CONDITION (RATIONAL METHOD)
- APPENDIX B.1: 100-YEAR HYDROLOGY CALCULATIONS (PROPOSED)
- APPENDIX C: ON-SITE HYDROLOGY BASED ON EXISTING CONDITION (UNIT HYDROGRAPH)
- APPENDIX C.1: 100-YEAR UNIT HYDROGRAPH CALCULATIONS (EXISTING)
- APPENDIX D: ON-SITE HYDROLOGY BASED ON PROPOSED CONDITION (UNIT HYDROGRAPH)
- APPENDIX D.1: 100-YEAR UNIT HYDROGRAPH CALCULATIONS (PROPOSED)
- APPENDIX E: REFERENCES
 - PLATE D-5.5: CURVE NUMBERS FOR PERVIOUS AREAS
 - NOAA ATLAS 14 PRECIPITATION DATA
 - NATIONAL RESOURCES CONSERVATION SERVICE WEB SOIL SURVEY
 - STORMTECH CUT SHEETS

EXHIBITS

EXHIBIT A:HYDROLOGY MAP – EXISTING CONDITION (RATIONAL)EXHIBIT B:HYDROLOGY MAP – PROPOSED CONDITION (RATIONAL)

I. PURPOSE AND SCOPE

The purpose of this drainage analysis is to quantify the 100-year storm event runoff emanating from the on-site drainage areas for APN 316-211-014, City of Moreno Valley, Riverside County, California. The study will analyze the existing and proposed hydrologic conditions of the Project's drainage areas and determine the necessary drainage improvements to convey the 100-year Project flows.

The scope of this analysis includes the following:

- 1. Determination of points of flow concentration and drainage areas.
- 2. Determination of the on-site 100-year peak storm flows based upon the existing and proposed conditions utilizing the Civil Design Software, Rational Tabling program for Riverside County.
- 3. Preparation of hydrology maps.
- 4. Preparation of the drainage report.

II. PROJECT DESCRIPTION

The proposed Project is located in the City of Moreno Valley, County of Riverside, California. The site is located along the east side of Heacock Street, south of Nandina Avenue. It is bounded by vacant lots to the north (APN 316-211-013) and to the east (APN 316-211-015). The south is bounded by Perris Valley Channel – Lateral B and a small lot (APN 316-211-016), and the west is bounded by Heacock Street. The existing boundary area is approximately 9.13 acres in size. The Project proposes to develop the property with a paved truck/trailer parking/storage lot which will include associated landscaping and street improvements. The final boundary area will be 8.88 acres in size with an additional dedication of right-of-way across Heacock Street.

III. DRAINAGE AREA OVERVIEW

Existing Condition

The project site is currently undeveloped. Topographically, site elevations range from approximately 1487 feet to 1482 feet above Mean Sea Level (MSL). The project drains from the northwest to the southeast to the neighboring property to the east (APN 316-211-015) at an approximate grade of 0.60%.

Proposed Condition

Upon development, the proposed drainage patterns will mimic the existing condition by sheet flowing from the northwest corner of the site to the southeast corner to a proposed catch basin. The catch basin will direct flows to an underground storage basin designed to mitigate increased flow volumes. A proposed sump and pump will pump flows from the

proposed underground basin to a proposed modular wetlands system designed for water quality purposes. Flows from the modular wetland will discharge via a storm drain line to the Riverside County Flood Control Channel to the south of the property.

IV. HYDROLOGY

The Riverside County Hydrology Manual (RCF&WCD), (Reference 1) was used to develop the hydrologic parameters for the hydrology analysis. Pre-development pervious areas will be analyzed as the "Barren" cover type per Plate D-5.5 of the RCF&WCD. Post-development pervious areas will be analyzed as the "Residential or Commercial Landscaping" cover type. In addition, Hydrologic Soil Groups (HSG) were determined using the Natural Resources Conservation Service Web Soil Survey (Reference 3). The study area consists of both HSG "A" and "B" (see Appendix E).

The Rational Method was used to determine the peak flow rates and times of concentration under the existing and proposed conditions. Computations were performed using the RSBC computer program developed by Civil Cadd/Civil Design Engineering Software.

V. **RESULTS**

Proposed and existing rational method results are summarized in Tables 5-1 and 5-2. Proposed and existing unit hydrograph method results are summarized in Tables 5-3 and 5-4.

		Existing	Condition	
Node	Drainag	e Area	Q ₁₀₀	TC_{100}
Node	(ft^2)	(ac)	(cfs)	(min)
102	387,027	8.88	13.38	22.15

Table 5-1: Existing Condition Rational Method Hydrology Results

		Proposed	Condition	
Node	Drainag	e Area	Q100	TC_{100}
Node	(ft^2)	(ac)	(cfs)	(min)
102	387,027	8.88	20.0	11.25

Table 5-2: Proposed Condition Rational Method Hydrology Results

D			Existing Condition	
Eve Freq./Du		Peak Flow Rate (cfs)	Flood Volume (cf)	Flood Volume (ac-ft)
100-yr	24-hr	4.23	61,505	1.41

 Table 5-3: Existing Condition Unit Hydrograph Method Hydrology Results

E			Proposed Condition	L
Eve Freq./Du		Peak Flow Rate (cfs)	Flood Volume (cf)	Flood Volume (ac-ft)
100-yr	24-hr	5.65	132,948	3.05

Table 5-4: Proposed Condition Unit Hydrograph Method Hydrology Results

Proposed Catch Basin (Node 102)

A catch basin is proposed at Node 102, which will convey the 100-year peak flow rate of 20.0 cfs from site flows to the proposed underground storage basin. The basin is designed to retain the difference in pre and post-development project flood volumes for the 100-year, 24 hour storm event. The proposed basin has a volume of 72,314 CF. A proposed sump and pump will pump flows to a proposed modular wetland for water quality purposes. Flows from the modular wetland will be directed south via a proposed storm drain to the Riverside County Flood Control Channel.

VI. STUDY FINDINGS

Upon development, the Project will generate a 100-year peak flow rate of 20.0 cfs to the proposed underground basin before being outletted to the south. The proposed pump and modular wetland will reduce post-development flows to be less than pre-development flows.

VII. CONCLUSION

Based on the findings of this analysis, the proposed grading and drainage designs are anticipated to protect the proposed on-site improvements from the 100-year storm event without causing adverse impacts to downstream drainage conditions. The project will be treated by a modular wetland system for water quality. The proposed basin will dewater via the proposed sump and pump.

VIII. REFERENCES

- 1. Riverside County; *Riverside County Flood Control & Water Conservation District Hydrology Manual*, April 1978.
- 2. NOAA's National Weather Service; NOAA Atlas 14, Volume 6, Version 2. May 12, 2021.
- 3. National Resources Conservation Service; Web Soil Survey. May 27, 2020.

FIGURE 1: REGIONAL VICINITY MAP

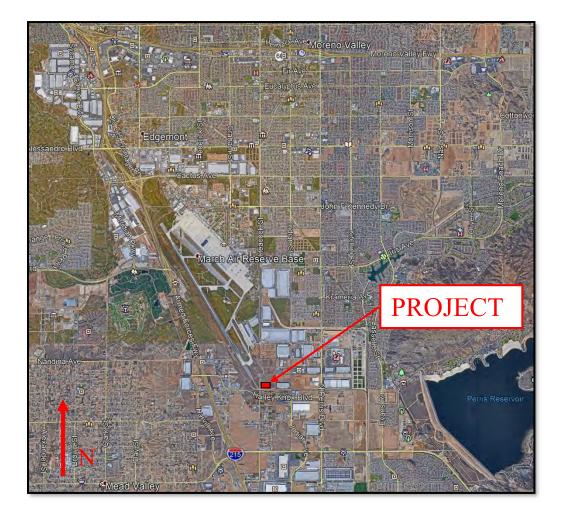


FIGURE 2: LOCAL VICINITY MAP



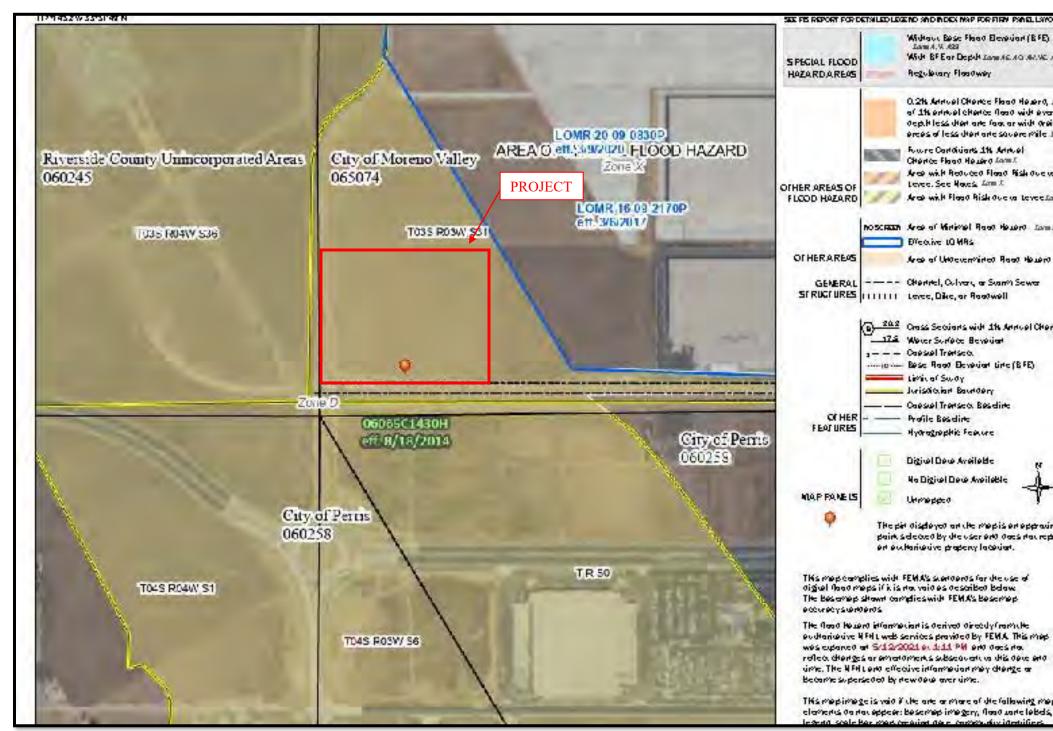


FIGURE 3: FEMA FLOODPLAIN MAP

PRELIMINARY DRAINAGE ANALYSIS **PROPOSED HEACOCK LOGISTICS PARKING LOT CITY OF MORENO VALLEY, CA**

NDEX MOP FOR FIRM PARELLISYOUT
. Base Flood Elemation (BFE)
V. A22 Ear Depth Long AC. ACC ANVE. AP
ary Flasowing
nnuel Cherros Flaco Helera, Arces Innuel Cherros Flaco Helera errogo :ss cherrore faco arwich Geinego Toss cherrore souere Mile Jam J
Cardivans 14 Annuel Fland Heldra Lamit In Reduced Fland Rish duc va ice Naces Lamit
h Flaso Aisk Ove ve Levee Lanso
Vinimel Roso Hezero . Low t c LO MAS
Unacientation Read Hellero Lorma
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caians with 1% Annual Chance unlace Bevalian Transeo. and Devalian Line (BFE) Sway
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Transco. Basidire Basidire
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APPENDIX A

ON-SITE HYDROLOGY BASED ON EXISTING CONDITION (RATIONAL METHOD)

APPENDIX A.1

100-YEAR HYDROLOGY CALCULATIONS (EXISTING)

Riverside County Rational Hydrology Program CIVILCADD/CIVILDESIGN Engineering Software, (c) 1989 - 2004 Version 7.0 Rational Hydrology Study Date: 08/03/21 File:14820001pr.out PRE-DEVELOPMENT DRAINAGE STUDY APN 316-211-014 - PROPOSED TRUCK/TRAILER PARKING/STORAGE LOT 100-YEAR STORM EVENT ******** Hydrology Study Control Information ********* English (in-1b) Units used in input data file Program License Serial Number 4042 Rational Method Hydrology Program based on Riverside County Flood Control & Water Conservation District 1978 hydrology manual Storm event (year) = 100.00 Antecedent Moisture Condition = 3 Standard intensity-duration curves data (Plate D-4.1) For the [Perris Valley] area used. 10 year storm 10 minute intensity = 1.880(In/Hr) 10 year storm 60 minute intensity = 0.780(In/Hr) 100 year storm 10 minute intensity = 2.690(In/Hr) 100 year storm 60 minute intensity = 1.120(In/Hr) Storm event year = 100.0 Calculated rainfall intensity data: 1 hour intensity = 1.120(In/Hr)Slope of intensity duration curve = 0.4900 ****** Process from Point/Station 101.000 to Point/Station 102.000 **** INITIAL AREA EVALUATION **** Initial area flow distance = 855.000(Ft.) Top (of initial area) elevation = 1487.000(Ft.) Bottom (of initial area) elevation = 1482.100(Ft.)

Difference in elevation = 4.900 (Ft.) Slope = 0.00573 s(percent) = 0.57TC = k(0.530)*[(length^3)/(elevation change)]^0.2 Initial area time of concentration = 22.152 min.

```
Rainfall intensity = 1.825(In/Hr) for a 100.0 year storm
UNDEVELOPED (poor cover) subarea
Runoff Coefficient = 0.826
Decimal fraction soil group A = 0.114
Decimal fraction soil group B = 0.886
Decimal fraction soil group C = 0.000
Decimal fraction soil group D = 0.000
RI index for soil(AMC 3) = 89.05
Pervious area fraction = 1.000; Impervious fraction = 0.000
Initial subarea runoff = 13.383(CFS)
Total initial stream area = 8.880(Ac.)
Pervious area fraction = 1,000
End of computations, total study area = 8.88 (Ac.)
The following figures may
be used for a unit hydrograph study of the same area.
Area averaged pervious area fraction(Ap) = 1.000
Area averaged RI index number = 76.7
```

APPENDIX B

ON-SITE HYDROLOGY BASED ON PROPOSED CONDITION (RATIONAL METHOD)

APPENDIX B.1

100-YEAR HYDROLOGY CALCULATIONS (PROPOSED)

Riverside County Rational Hydrology Program

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CIVILCADD/CIVILDESIGN Engineering Software, (c) 1989 - 2004 Version 7.0
 Rational Hydrology Study Date: 05/12/21 File:14820001PO.out
POST-DEVELOPMENT DRAINAGE STUDY
APN 316-211-014 - PROPOSED TRUCK/TRAILER PARKING/STORAGE LOT
100-YEAR STORM EVENT
 ******* Hydrology Study Control Information *********
English (in-lb) Units used in input data file
Program License Serial Number 4042
    Rational Method Hydrology Program based on
Riverside County Flood Control & Water Conservation District
1978 hydrology manual
Storm event (year) = 100.00 Antecedent Moisture Condition = 3
Standard intensity-duration curves data (Plate D-4.1)
For the [ Perris Valley ] area used.
10 year storm 10 minute intensity = 1.880(In/Hr)
10 year storm 60 minute intensity = 0.780(In/Hr)
100 year storm 10 minute intensity = 2.690(In/Hr)
100 year storm 60 minute intensity = 1.120(In/Hr)
Storm event year = 100.0
Calculated rainfall intensity data:
1 hour intensity = 1.120(In/Hr)
Slope of intensity duration curve = 0.4900
Process from Point/Station 101.000 to Point/Station 102.000
**** INITIAL AREA EVALUATION ****
Initial area flow distance = 850.000(Ft.)
Top (of initial area) elevation = 1489.700(Ft.)
Bottom (of initial area) elevation = 1481.400(Ft.)
Difference in elevation = 8.300(Ft.)
Slope = 0.00976 s(percent) = 0.98
TC = k(0.300)*[(length^3)/(elevation change)]^0.2
Initial area time of concentration = 11.245 min.
Rainfall intensity = 2.544 (In/Hr) for a 100.0 year storm
COMMERCIAL subarea type
Runoff Coefficient = 0.885
Decimal fraction soil group A = 0.114
```

Decimal fraction soil group B = 0.886Decimal fraction soil group C = 0.000Decimal fraction soil group D = 0.000RI index for soil(AMC 3) = 72.61 Pervious area fraction = 0.100; Impervious fraction = 0.900 Initial subarea runoff = 19.997(CFS) Total initial stream area = 8.880(Ac.) Pervious area fraction = 0.100 End of computations, total study area = 8.88 (Ac.) The following figures may be used for a unit hydrograph study of the same area. Area averaged pervious area fraction(Ap) = 0.100

Area averaged RI index number = 53.3

APPENDIX C

ON-SITE HYDROLOGY BASED ON EXISTING CONDITION (UNIT HYDROGRAPH)

APPENDIX C.1

100-YEAR UNIT HYDROGRAPH CALCULATIONS (EXISTING)

```
Unit Hydrograph Analysis
        Copyright (c) CIVILCADD/CIVILDESIGN, 1989 - 2004, Version 7.0
             Study date 08/03/21 File: 14820001prun24100.out
    Riverside County Synthetic Unit Hydrology Method
    RCFC & WCD Manual date - April 1978
    Program License Serial Number 4042
           _____
    English (in-lb) Input Units Used
     English Rainfall Data (Inches) Input Values Used
     English Units used in output format
                            _____
    PRE-DEVELOPMENT
    APN 316-211-014 - PROPOSED TRUCK/TRAILER PARKING/STORAGE LOT
    100-YEAR STORM EVENT
    Drainage Area = 8.88(Ac.) = 0.014 Sq. Mi.
    Drainage Area for Depth-Area Areal Adjustment = 8.88(Ac.) =
0.014 Sq. Mi.
    Length along longest watercourse = 855.00(Ft.)
    Length along longest watercourse measured to centroid = 88.00(Ft.)
    Length along longest watercourse = 0.162 Mi.
    Length along longest watercourse measured to centroid = 0.017 Mi.
    Difference in elevation = 4.90(Ft.)
Slope along watercourse = 30.2596 Ft./Mi.
    Average Manning's 'N' = 0.030
    Lag time = 0.040 Hr.
    Lag time = 2.39 Min.
    25% of lag time = 0.60 Min.
    40% of lag time = 0.96 Min.
    Unit time = 5.00 Min.
    Duration of storm = 24 Hour(s)
    User Entered Base Flow = 0.00(CFS)
    2 YEAR Area rainfall data:
    Area(Ac.)[1] Rainfall(In)[2] Weighting[1*2]
8.88 1.89 16.78
```

100 YEAR Area rainfall data: Area(Ac.)[1] Rainfall(In)[2] Weighting[1*2] 8.88 4.82 42.80 STORM EVENT (YEAR) = 100.00 Area Averaged 2-Year Rainfall = 1.890(In) Area Averaged 100-Year Rainfall = 4.820(In) Point rain (area averaged) = 4.820(In) Areal adjustment factor = 100.00 % Adjusted average point rain = 4.820(In) Sub-Area Data: Area(Ac.) Runoff Index Impervious % 0.000

 Area(Ac.)
 Runoff Index
 Impervious %

 7.870
 86.00
 0.000

 1.010
 78.00
 0.000

 Total Area Entered =
 8.88(Ac.)

 RI
 RI
 Infil. Rate Impervious
 Adj. Infil. Rate Area%
 F

 AMC2
 AMC-3
 (In/Hr)
 (Dec.%)
 (In/Hr)
 (Dec.)
 (In/Hr)

 86.0
 94.4
 0.073
 0.000
 0.073
 0.886
 0.065
 86.094.40.0730.0000.0730.8860.06578.089.80.1320.0000.1320.1140.015 Sum (F) = 0.080Area averaged mean soil loss (F) (In/Hr) = 0.213 Minimum soil loss rate ((In/Hr)) = 0.106 (for 24 hour storm duration) Note: User entry of the f value Soil low loss rate (decimal) = 0,900 ____ Unit Hydrograph VALLEY S-Curve _____ Unit Hydrograph Data _____ ____ Unit time period Time % of lag Distribution Unit Hydrograph Graph % (CFS) (hrs) _____ -----_____
 Unit Time
 Pattern
 Storm Rain
 Loss rate(In./Hr)
 Effective

 (Hr.)
 Percent
 (In/Hr)
 Max
 Low
 (In/Hr)

 1
 0.08
 0.07
 0.039
 0.378
 0.035
 0.00

 2
 0.17
 0.07
 0.039
 0.376
 0.035
 0.00

 3
 0.25
 0.07
 0.039
 0.375
 0.035
 0.00

4	0.33	0.10	0.058	0.373	0,052	0.01
5	0.42	0.10	0.058	0.372	0.052	0.01
6	0.50	0.10	0.058	0.370	0.052	0.01
7	0.58	0.10	0.058	0.369	0.052	0.01
8	0.67	0.10	0.058	0.367	0.052	0.01
9	0.75	0.10	0.058	0.366	0.052	0.01
10	0.83	0.13	0.077	0.364	0.069	0.01
11	0.92	0.13	0.077	0.363	0.069	0.01
12	1.00	0.13	0.077	0.362	0.069	0.01
13	1.08	0.10	0.058	0.360	0.052	0.01
14	1.17	0.10	0.058	0.359	0.052	0.01
14	1.25	0.10	0.058	0.357	0.052	0.01
16	1.23	0.10	0.058	0.356	0.052	0.01
	1.42	0.10	0.058	0.355	0.052	0.01
17	1.50	0.10	0.058	0.353	0.052	0.01
18			0.058	0.352	0.052	0.01
19	1.58	0.10	0.058	0.350	0.052	0.01
20	1.67	0.10		0.349	0.052	0.01
21	1.75	0.10	0.058		0.069	0.01
22	1.83	0.13	0.077	0.347	0.069	0.01
23	1.92	0.13	0.077	0.346	0.069	0.01
24	2.00	0.13	0.077	0.345		0.01
25	2.08	0.13	0.077	0.343	0.069	
26	2.17	0.13	0.077	0.342	0.069	0.01
27	2.25	0.13	0.077	0.340	0.069	0.01
28	2.33	0.13	0.077	0.339	0.069	0.01
29	2.42	0.13	0.077	0.338	0.069	0.01
30	2.50	0.13	0.077	0.336	0.069	0.01
31	2.58	0.17	0.096	0.335	0.087	0.01
32	2.67	0.17	0.096	0.334	0.087	0.01
33	2.75	0.17	0.096	0.332	0.087	0.01
34	2.83	0.17	0.096	0.331	0.087	0.01
35	2.92	0.17	0.096	0.329	0.087	0.01
36	3.00	0.17	0.096	0.328	0.087	0.01
37	3.08	0.17	0.096	0.327	0.087	0.01
38	3.17	0.17	0.096	0.325	0.087	0.01
39	3.25	0.17	0.096	0.324	0.087	0.01
40	3.33	0.17	0.096	0.323	0.087	0.01
41	3.42	0.17	0.096	0.321	0.087	0.01
42	3.50	0.17	0.096	0.320	0.087	0.01
43	3.58	0.17	0.096	0.319	0.087	0.01
44	3.67	0.17	0.096	0.317	0.087	0.01
45	3.75	0.17	0.096	0.316	0.087	0.01
46	3.83	0.20	0.116	0.315	0.104	0.01
47	3.92	0.20	0.116	0.313	0.104	0.01
48	4.00	0.20	0.116	0.312	0.104	0.01
49	4.08	0.20	0.116	0.311	0.104	0.01
50	4.17	0.20	0.116	0.309	0.104	0.01
51	4.25	0.20	0.116	0.308	0.104	0.01
52	4.33	0.23	0.135	0.307	0.121	0.01
53	4.42	0.23	0.135	0.305	0.121	0.01
54	4.50	0.23	0.135	0.304	0.121	0.01
55	4.58	0.23	0.135	0.303	0.121	0.01
56	4.67	0.23	0.135	0.302	0.121	0.01
	4.75	0.23	0.135	0.300	0.121	0.01

4.83 4.92 5.00 5.08 5.17 5.25 5.33 5.42 5.50 5.58	0.27 0.27 0.20 0.20 0.20 0.20 0.20 0.23 0.23	0.154 0.154 0.154 0.116 0.116 0.116	0.299 0.298 0.296 0.295 0.294	0.139 0.139 0.139 0.104	0.02 0.02 0.02 0.01
4.92 5.00 5.08 5.17 5.25 5.33 5.42 5.50	0.27 0.27 0.20 0.20 0.20 0.20 0.23	0.154 0.154 0.116 0.116 0.116	0.298 0.296 0.295	0.139 0.139 0.104	0.02 0.01
5.00 5.08 5.17 5.25 5.33 5.42 5.50	0.27 0.20 0.20 0.20 0.20 0.23	0.154 0.116 0.116 0.116	0.296 0.295	0.139 0.104	0.01
5.08 5.17 5.25 5.33 5.42 5.50	0.20 0.20 0.20 0.23	0.116 0.116 0.116	0.295		
5.17 5.25 5.33 5.42 5.50	0.20 0.20 0.23	0.116 0.116	0.294	0 104	the second se
5.25 5.33 5.42 5.50	0.20	0.116		0.104	0.01
5.33 5.42 5.50	0.23		0.292	0.104	0.01
5.42 5.50		0.135	0.291	0.121	0.01
5.50	V * 60 - 0	0.135	0.290	0.121	0.01
	0.23	0.135	0.289	0.121	0.01
0.00	0.27	0.154	0.287	0.139	0,02
5.67	0.27	0.154	0.286	0.139	0.02
5.75	0.27	0.154	0.285	0.139	0.02
5,83	0.27	0.154			0.02
5,92	0.27	0.154			0.02
6.00	0.27				0.02
6.08					0.02
6.17					0.02
6.25					0.02
					0.02
					0.02
					0.02
					0.02
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					0.04
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				101-	0.04
					0.04
					0.04
				222	0.04
					0.06
					0.07
					0.07
					0.09
					0.09
					0.09
					0.13
			0.236		0.13
	0.63		0.235		0.13
	5,92 6.00 6.08 6.17	5.92 0.27 6.00 0.27 6.08 0.30 6.17 0.30 6.25 0.30 6.33 0.30 6.42 0.30 6.50 0.33 6.70 0.33 6.75 0.33 6.75 0.33 6.75 0.33 7.00 0.33 7.00 0.33 7.17 0.33 7.25 0.33 7.33 0.37 7.42 0.37 7.50 0.37 7.50 0.40 7.67 0.40 7.75 0.40 7.67 0.40 7.67 0.40 7.50 0.50 8.17 0.50 8.17 0.50 8.25 0.50 8.33 0.50 8.42 0.50 8.50 0.53 8.67 0.53 8.75 0.53 8.83 0.57 9.00 0.57 9.08 0.63 9.17 0.63	5.92 0.27 0.154 6.00 0.27 0.154 6.08 0.30 0.174 6.17 0.30 0.174 6.25 0.30 0.174 6.33 0.30 0.174 6.42 0.30 0.174 6.50 0.30 0.174 6.50 0.30 0.174 6.50 0.30 0.174 6.50 0.30 0.174 6.50 0.30 0.174 6.50 0.33 0.193 6.67 0.33 0.193 6.75 0.33 0.193 6.92 0.33 0.193 7.00 0.33 0.193 7.00 0.33 0.193 7.17 0.33 0.193 7.25 0.33 0.193 7.17 0.33 0.193 7.17 0.33 0.193 7.17 0.33 0.193 7.17 0.33 0.193 7.17 0.33 0.193 7.17 0.37 0.212 7.50 0.37 0.212 7.50 0.37 0.212 7.50 0.40 0.231 7.67 0.40 0.231 7.67 0.40 0.231 7.92 0.43 0.251 8.00 0.43 0.251 8.00 0.50 0.289 8.17 0.50 0.289 8.25 0.50 0.289 8.58 0.53 0.308 <t< td=""><td>5.92$0.27$$0.154$$0.282$$6.00$$0.27$$0.154$$0.281$$6.08$$0.30$$0.174$$0.280$$6.17$$0.30$$0.174$$0.279$$6.25$$0.30$$0.174$$0.277$$6.33$$0.30$$0.174$$0.277$$6.42$$0.30$$0.174$$0.275$$6.50$$0.30$$0.174$$0.274$$6.58$$0.33$$0.193$$0.272$$6.67$$0.33$$0.193$$0.271$$6.75$$0.33$$0.193$$0.269$$6.92$$0.33$$0.193$$0.266$$7.00$$0.33$$0.193$$0.266$$7.00$$0.33$$0.193$$0.266$$7.17$$0.33$$0.193$$0.266$$7.25$$0.33$$0.193$$0.2663$$7.33$$0.37$$0.212$$0.262$$7.42$$0.37$$0.212$$0.262$$7.42$$0.37$$0.212$$0.257$$7.5$$0.40$$0.231$$0.255$$7.67$$0.40$$0.231$$0.257$$7.75$$0.40$$0.251$$0.252$$8.08$$0.50$$0.289$$0.251$$8.17$$0.53$$0.308$$0.244$$8.50$$0.50$$0.289$$0.244$$8.50$$0.50$$0.289$$0.244$$8.50$$0.53$$0.308$$0.244$$8.67$$0.53$$0.308$$0.244$$8.67$$0.53$$0.328$<td>5.92$0.27$$0.154$$0.282$$0.139$$6.00$$0.27$$0.154$$0.281$$0.139$$6.08$$0.30$$0.174$$0.279$$0.156$$6.17$$0.30$$0.174$$0.277$$0.156$$6.25$$0.30$$0.174$$0.277$$0.156$$6.33$$0.30$$0.174$$0.276$$0.156$$6.42$$0.30$$0.174$$0.272$$0.156$$6.50$$0.30$$0.174$$0.272$$0.174$$6.75$$0.33$$0.193$$0.271$$0.174$$6.75$$0.33$$0.193$$0.270$$0.174$$6.83$$0.33$$0.193$$0.269$$0.174$$6.92$$0.33$$0.193$$0.266$$0.174$$7.08$$0.33$$0.193$$0.266$$0.174$$7.17$$0.33$$0.193$$0.266$$0.174$$7.25$$0.33$$0.193$$0.266$$0.174$$7.33$$0.37$$0.212$$0.262$$0.191$$7.42$$0.37$$0.212$$0.262$$0.191$$7.50$$0.40$$0.231$$0.256$$0.208$$7.75$$0.40$$0.231$$0.256$$0.208$$7.83$$0.43$$0.251$$0.252$$0.226$$7.92$$0.43$$0.251$$0.252$$0.226$$7.92$$0.43$$0.251$$0.252$$0.226$$7.92$$0.43$$0.251$$0.252$$0.226$$7.92$$0.43$<t< td=""></t<></td></td></t<>	5.92 0.27 0.154 0.282 6.00 0.27 0.154 0.281 6.08 0.30 0.174 0.280 6.17 0.30 0.174 0.279 6.25 0.30 0.174 0.277 6.33 0.30 0.174 0.277 6.42 0.30 0.174 0.275 6.50 0.30 0.174 0.274 6.58 0.33 0.193 0.272 6.67 0.33 0.193 0.271 6.75 0.33 0.193 0.269 6.92 0.33 0.193 0.266 7.00 0.33 0.193 0.266 7.00 0.33 0.193 0.266 7.17 0.33 0.193 0.266 7.25 0.33 0.193 0.2663 7.33 0.37 0.212 0.262 7.42 0.37 0.212 0.262 7.42 0.37 0.212 0.257 7.5 0.40 0.231 0.255 7.67 0.40 0.231 0.257 7.75 0.40 0.251 0.252 8.08 0.50 0.289 0.251 8.17 0.53 0.308 0.244 8.50 0.50 0.289 0.244 8.50 0.50 0.289 0.244 8.50 0.53 0.308 0.244 8.67 0.53 0.308 0.244 8.67 0.53 0.328 <td>5.92$0.27$$0.154$$0.282$$0.139$$6.00$$0.27$$0.154$$0.281$$0.139$$6.08$$0.30$$0.174$$0.279$$0.156$$6.17$$0.30$$0.174$$0.277$$0.156$$6.25$$0.30$$0.174$$0.277$$0.156$$6.33$$0.30$$0.174$$0.276$$0.156$$6.42$$0.30$$0.174$$0.272$$0.156$$6.50$$0.30$$0.174$$0.272$$0.174$$6.75$$0.33$$0.193$$0.271$$0.174$$6.75$$0.33$$0.193$$0.270$$0.174$$6.83$$0.33$$0.193$$0.269$$0.174$$6.92$$0.33$$0.193$$0.266$$0.174$$7.08$$0.33$$0.193$$0.266$$0.174$$7.17$$0.33$$0.193$$0.266$$0.174$$7.25$$0.33$$0.193$$0.266$$0.174$$7.33$$0.37$$0.212$$0.262$$0.191$$7.42$$0.37$$0.212$$0.262$$0.191$$7.50$$0.40$$0.231$$0.256$$0.208$$7.75$$0.40$$0.231$$0.256$$0.208$$7.83$$0.43$$0.251$$0.252$$0.226$$7.92$$0.43$$0.251$$0.252$$0.226$$7.92$$0.43$$0.251$$0.252$$0.226$$7.92$$0.43$$0.251$$0.252$$0.226$$7.92$$0.43$<t< td=""></t<></td>	5.92 0.27 0.154 0.282 0.139 6.00 0.27 0.154 0.281 0.139 6.08 0.30 0.174 0.279 0.156 6.17 0.30 0.174 0.277 0.156 6.25 0.30 0.174 0.277 0.156 6.33 0.30 0.174 0.276 0.156 6.42 0.30 0.174 0.272 0.156 6.50 0.30 0.174 0.272 0.174 6.75 0.33 0.193 0.271 0.174 6.75 0.33 0.193 0.270 0.174 6.83 0.33 0.193 0.269 0.174 6.92 0.33 0.193 0.266 0.174 7.08 0.33 0.193 0.266 0.174 7.17 0.33 0.193 0.266 0.174 7.25 0.33 0.193 0.266 0.174 7.33 0.37 0.212 0.262 0.191 7.42 0.37 0.212 0.262 0.191 7.50 0.40 0.231 0.256 0.208 7.75 0.40 0.231 0.256 0.208 7.83 0.43 0.251 0.252 0.226 7.92 0.43 0.251 0.252 0.226 7.92 0.43 0.251 0.252 0.226 7.92 0.43 0.251 0.252 0.226 7.92 0.43 <t< td=""></t<>

				a succession		4.55
112	9.33	0.67	0.386	0.234		0.15
113	9.42	0.67	0.386	0.233		0.15
114	9.50	0.67	0.386	0.231		0.15
115	9.58	0.70	0.405	0.230		0.17
116	9.67	0.70	0.405	0.229		0.18
117	9.75	0.70	0.405	0.228		0.18
118	9.83	0.73	0.424	0.227		0.20
119	9.92	0.73	0.424	0.226		0.20
120	10.00	0.73	0.424	0.225		0.20
121	10.08	0.50	0.289	0.224		0.07
122	10.17	0.50	0.289	0.223		0.07
123	10.25	0.50	0.289	0.222		0.07
124	10.33	0.50	0.289	0.221		0.07
125	10.42	0.50	0.289	0.219	0.000	0.07
126	10.50	0.50	0.289	0.218		0.07
127	10.58	0.67	0.386	0.217		0.17
128	10.67	0.67	0.386	0.216		0.17
129	10.75	0.67	0.386	0.215		0.17
130	10.83	0.67	0.386	0.214		0.17
131	10.92	0.67	0.386	0.213		0.17
132	11.00	0.67	0.386	0.212		0.17
133	11.08	0.63	0.366	0.211		0.16
134	11.17	0.63	0.366	0.210		0.16
135	11.25	0.63	0.366	0.209		0.16
136	11.33	0.63	0.366	0.208		0.16
137	11.42	0.63	0.366	0.207		0.16
138	11.50	0.63	0.366	0.206		0.16
139	11,58	0.57	0.328	0.205		0.12
140	11.67	0.57	0.328	0.204		0.12
141	11.75	0.57	0.328	0.203		0.12
142	11.83	0.60	0.347	0.202		0.15
143	11.92	0.60	0.347	0.201		0.15
144	12.00	0.60	0.347	0.200	++++	0.15
145	12.08	0.83	0.482	0.199		0.28
		0.83	0.482	0.198		0.28
146	12.17		0.482	0.197		0.29
147	12.25	0.83	0.501	0.196		0.31
148	12.33	0.87		0.195		0.31
149	12.42	0.87	0.501			0.31
150	12.50	0.87	0.501	0.194	1223	0.35
151	12.58	0.93	0.540	0.193 0.192	000	0.35
152	12.67	0.93	0.540		1000	0.35
153	12.75	0.93	0.540	0.191	000	0.37
154	12.83	0.97	0.559	0.190	222	0.37
155	12.92	0.97	0.559	0.189		
156	13.00	0.97	0.559	0.188		0.37
157	13.08	1.13	0.656	0.187	222	0.47
158	13.17	1.13	0.656	0.186		0.47
159	13.25	1,13	0.656	0.185		0.47
160	13.33	1.13	0.656	0.184		0.47
161	13.42	1.13	0.656	0.183		0.47
162	13.50	1.13	0.656	0.182	777	0.47
163	13.58	0.77	0.443	0.181		0.26
164	13.67	0.77	0.443	0.181		0.26
165	13.75	0.77	0.443	0.180	1461 H	0.26

166	13.83	0.77	0.443	0,179		0.26
167	13.92	0.77	0.443	0.178		0.27
168	14.00	0.77	0.443	0.177		0.27
169	14.08	0.90	0.521	0.176		0.34
170	14.17	0.90	0.521	0.175		0.35
171	14.25	0.90	0.521	0.174		0.35
172	14.33	0.87	0.501	0.173		0.33
173	14.42	0.87	0.501	0.172		0.33
174	14.50	0.87	0.501	0.172		0.33
175	14.58	0.87	0.501	0.171		0.33
176	14.67	0.87	0.501	0.170	\rightarrow \leftrightarrow \rightarrow	0.33
177	14.75	0.87	0.501	0.169		0.33
178	14.83	0.83	0.482	0.168	+++	0.31
179	14.92	0.83	0.482	0.167		0.31
180	15.00	0.83	0.482	0.166		0.32
181	15.08	0.80	0.463	0.165	++++	0.30
182	15.17	0.80	0.463	0.165		0.30
183	15.25	0.80	0.463	0.164		0.30
184	15.33	0.77	0.443	0.163		0.28
185	15.42	0.77	0.443	0,162		0.28
186	15.50	0.77	0.443	0.161		0.28
187	15.58	0.63	0.366	0.160		0.21
188	15.67	0.63	0.366	0.160		0.21
189	15.75	0.63	0.366	0.159		0.21
190	15.83	0.63	0.366	0.158		0.21
191	15.92	0.63	0.366	0.157	\rightarrow \leftrightarrow \leftarrow	0.21
192	16.00	0.63	0.366	0.156		0.21
193	16.08	0.13	0.077	0.156	0.069	0.01
194	16.17	0.13	0.077	0.155	0.069	0.01
195	16.25	0.13	0.077	0.154	0.069	0.01
196	16.33	0.13	0.077	0.153	0.069	0.01
197	16.42	0.13	0.077	0.152	0.069	0.01
198	16.50	0.13	0.077	0.152	0.069	0.01
199	16.58	0.10	0.058	0.151	0.052	0.01
200	16.67	0.10	0.058	0.150	0.052	0.01
201	16.75	0.10	0.058	0.149	0.052	0.01
202	16.83	0.10	0.058	0.149	0.052	0.01
203	16.92	0.10	0.058	0.148	0.052	0.01
204	17.00	0.10	0.058	0.147	0.052	0.01
205	17.08	0.17	0.096	0.146	0.087	0.01
206	17.17	0.17	0.096	0.146	0.087	0.01
207	17.25	0.17	0.096	0.145	0.087	0.01
208	17.33	0.17	0.096	0.144	0.087	0.01
209	17.42	0.17	0.096	0.143	0.087	0.01
210	17.50	0.17	0.096	0.143	0.087	0.01
211	17,58	0.17	0.096	0.142 0.141	0.087	0.01
212	17.67	0.17	0.096 0.096	0.141	0.087	0.01 0.01
213	17.75	0.17		0.140	0.069	0.01
214	17.83	0.13	0.077	0.139	0.069	0.01
215 216	17.92 18,00	0.13	0.077 0.077	0.139	0.069	0.01
216	18.00	0.13	0.077	0.139	0.069	0.01
217	18.08 18.17	0.13	0.077	0.137	0.069	0.01
218	18.17	0.13	0.077	0.136	0.069	0.01
413	10.20	0.13	0.017	0.100	0.000	0.01

220	18.33	0.13	0.077	0.136	0.069	0.01
221	18.42	0.13	0.077	0.135	0.069	0.01
222	18.50	0.13	0.077	0.135	0.069	0.01
223	18.58	0.10	0.058	0.134	0.052	0.01
224	18.67	0.10	0.058	0.133	0.052	0.01
225	18.75	0.10	0.058	0.133		0.01
226	18.83	0.07	0.039	0.132	0.035	0.00
227	18,92	0.07	0.039	0.131	0.035	0.00
228	19.00	0.07	0.039	0.131	0.035	0.00
229	19.08	0.10	0.058		0.052	0.01
230	19.17	0.10	0.058	0.129		0.01
231	19.25		0.058	0.129		0.01
232	19.33		0.077	0.128		
	19.33		0.077	0.128		0.01
233		0.13		0.127		0.01
234			0.058	0.126		0.01
235	19.58			0.126		
236		0.10	0.058	0.125		
237		0.10				
238	19.83		0.039	0.125		
239	19.92		0.039			0.00
240	20.00		0.039			0.00
241	20.08	0.10	0.058			0.01
242	20.17		0.058	0.123		0.01
243	20.25	0.10		0.122		0.01
244	20.33			0.122		0.01
245	20.42		0.058	0.121		0.01
246	20.50	0.10		0.120		0.01
247	20.58	0.10	0.058	0.120		0.01
248	20.67	0.10	0.058	0.119		0.01
249	20.75	0.10	0.058	0.119	0.052	0.01
250	20.83	0.07	0.039	0.118	0.035	0.00
251	20.92	0.07	0.039	0.118		0.00
252	21.00	0.07	0.039	0.118	0.035	0.00
253	21.08	0.10	0.058	0.117	0.052	0.01
254	21.17	0.10	0.058	0.117	0.052	0.01
255	21.25	0.10	0.058	0.116	0.052	0.01
256	21,33	0.07	0.039	0.116	0.035	0.00
257	21.42	0.07	0.039	0.115	0.035	0.00
258	21,50	0.07	0.039	0.115	0.035	0.00
	21.58	0.10	0.058	0.114	0.052	0.01
260	21.67	0.10	0.058	0.114	0.052	0.01
261	21.75	0.10	0.058	0.114	0.052	0.01
262	21,83	0.07	0.039	0.113	0.035	0.00
263	21.92	0.07	0.039	0.113	0.035	0.00
264	22.00	0.07	0.039	0.112	0.035	0.00
265	22.08	0.10	0.058	0.112	0.052	0.01
266	22.17	0.10	0.058	0.112	0.052	0.01
		0.10	0.058	0.111	0.052	0.01
267	22.25			0.111	0.035	0.00
268	22.33	0.07	0.039	0.111	0.035	0.00
269	22.42	0.07	0.039		0.035	0.00
270	22.50	0.07	0.039	0.110		0.00
271	22.58	0.07	0.039	0.110	0.035	
272	22.67	0.07	0.039	0.110 0.109	0.035 0.035	0.00
273	22.75	0.07	0.039	0 0 9	0.035	0.00

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74	22.83	0.07	0.039	0.109	0.035	0.00	
75	22.92		0.039		0.035	0.00	
76	23.00		0.039	0 109	0.035	0.00	
		0.07	0.039	0.109		0.00	
77	23.08	0.4 0.5		0.100	0.035	0.00	
78	23.17	0.07	0.039	0.108	0.035	0.00	
79	23.25	0.07	0.039	0.108	0.035	0.00	
80	23.33	0.07	0.039	0.108	0,035	0.00	
81	23.42	0.07	0.039	0.107	0.035	0.00	
82	23.50	0.07	0 0 2 0		0.035		
83	23.58	0.07	0.039	0.107	0.035	0.00	
84	23.67	0.07	0.039	0.107	0.035	0.00	
85	23.75	0.07 0.07 0.07 0.07 0.07 0.07 0.07	0.039	0.107	0.035	0.00	
86	23 83	0.07	0.039		0.035		
87	23.92	0.07	0.039	0 107	0.035	0.00	
	23.34	0.07	0.029		0.035		
88	24.00	0.07	0.039	0.100	0.035	22 0	
	Sum =	100.0	and a star		Sum -	22.9	
	Flood	volume = Effect	ctive rainfa	1.5	(1n)	121.1	
	times	s area 8.	.9(Ac.)/[(In	(Ft.)] =	1.4(Ac	.Ft)	
	Total	soil loss =	2.91(In)				
	Total	soil loss =	2.155 (Ac.	Ft)			
	Total	rainfall =	4.82(In)				
	Flood	volume =	61504.9 Cu	bic Feet			
		soil loss =					
	a a server						in the last set.
	Peak		this hydrogr ++++++++++++ 24 - H O U n o f f	aph = +++++++++++ R S T O H y d r o	4.230(CFS) ++++++++++++ R M g r a p h		+++
	Peak +++++	flow rate of t +++++++++++++++ R u n Hydrogra	this hydrogr ++++++++++ 24 - H O U n o f f aph in 5	<pre>caph =</pre>	4.230(CFS) +++++++++++ R M g r a p h cervals ((CFS))	
Tir	Peak +++++-	flow rate of t	this hydrogr ++++++++++ 24 - H O U n o f f aph in 5 Q(CFS) 0	<pre>t+t+t+t+t+t R S T O H y d r o Minute int 2.5</pre>	4.230(CFS) +++++++++++ R M g r a p h cervals ((CFS		 10.1
-	Peak +++++ ne (h+m)	flow rate of t +++++++++++++ R u n Hydrogra Volume Ac.Ft	this hydrogr ++++++++++ 24 - H O U n o f f aph in 5 Q(CFS) 0	<pre>t+t+t+t+t+t R S T O H y d r o Minute int 2.5</pre>	4.230(CFS) +++++++++++ R M g r a p h cervals ((CFS))	
(Peak +++++ ne(h+m) 0+ 5	flow rate of t ++++++++++++++++++++++++++++++++++++	this hydrogr ++++++++++ 24 - H O U n o f f aph in 5 Q(CFS) 0 0.02 Q	aph = ++++++++++ R S T O H y d r o Minute int 2.5	4.230(CFS) +++++++++++ R M g r a p h cervals ((CFS))	
(Peak +++++ me(h+m) 0+ 5 0+10	flow rate of the second	this hydrogr ++++++++++ 24 - H O U n o f f aph in 5 Q(CFS) 0 0.02 Q 0.03 Q	aph = ++++++++++ R S T O H y d r o Minute int 2.5	4.230(CFS) +++++++++++ R M g r a p h cervals ((CFS))	
	Peak +++++ me(h+m) 0+ 5 0+10 0+15	flow rate of t ++++++++++++ R u n Hydrogra Volume Ac.Ft 0.0001 0.0003 0.0005	this hydrogr ++++++++++ 24 - H O U n o f f aph in 5 Q(CFS) 0 0.02 Q 0.03 Q 0.03 Q 0.03 Q	aph = ++++++++++ R S T O H y d r o Minute int 2.5	4.230(CFS) +++++++++++ R M g r a p h cervals ((CFS))	
	Peak +++++ me(h+m) 0+ 5 0+10 0+15 0+20	flow rate of t ++++++++++++++++++++++++++++++++++++	this hydrogr ++++++++++ 24 - H O U n o f f aph in 5 Q(CFS) 0 0.02 Q 0.03 Q 0.03 Q 0.03 Q 0.03 Q	aph = ++++++++++ R S T O H y d r o Minute int 2.5	4.230(CFS) +++++++++++ R M g r a p h cervals ((CFS))	
	Peak +++++ me(h+m) 0+ 5 0+10 0+15 0+20 0+25	flow rate of t ++++++++++++++++++++++++++++++++++++	this hydrogr ++++++++++ 24 - H O U n o f f 2000 0 0.02 Q 0.03 Q 0.03 Q 0.03 Q 0.03 Q 0.04 Q 0.05 Q	aph = ++++++++++ R S T O H y d r o Minute int 2.5	4.230(CFS) +++++++++++ R M g r a p h cervals ((CFS))	
 ((((((Peak +++++ me(h+m) me(h+m) 0+ 5 0+10 0+15 0+20 0+25 0+20 0+25 0+30	flow rate of t ++++++++++++++++++++++++++++++++++++	this hydrogr ++++++++++ 24 - H O U n o f f 2000 Q 0.02 Q 0.03 Q 0.03 Q 0.03 Q 0.03 Q 0.04 Q 0.05 Q 0.05 Q	aph = ++++++++++ R S T O H y d r o Minute int 2.5	4.230(CFS) +++++++++++ R M g r a p h cervals ((CFS))	
	Peak +++++ me(h+m) 0+ 5 0+10 0+15 0+20 0+25 0+30 0+35	flow rate of t ++++++++++++++++++++++++++++++++++++	this hydrogr +++++++++++ 24 - H O U n o f f Q(CFS) 0 0.02 Q 0.03 Q 0.03 Q 0.03 Q 0.03 Q 0.03 Q 0.04 Q 0.05 Q 0.05 Q	aph = ++++++++++ R S T O H y d r o Minute int 2.5	4.230(CFS) +++++++++++ R M g r a p h cervals ((CFS))	
	Peak +++++ me(h+m) 0+ 5 0+10 0+15 0+20 0+25 0+20 0+25 0+30 0+35 0+40	flow rate of t ++++++++++++++++++++++++++++++++++++	this hydrogr +++++++++++ 24 - H O U n o f f Q(CFS) 0 0.02 Q 0.03 Q 0.03 Q 0.03 Q 0.03 Q 0.03 Q 0.04 Q 0.05 Q 0.05 Q 0.05 Q	aph = ++++++++++ R S T O H y d r o Minute int 2.5	4.230(CFS) +++++++++++ R M g r a p h cervals ((CFS))	
	Peak +++++ me(h+m) 0+ 5 0+10 0+15 0+20 0+25 0+20 0+25 0+30 0+35 0+40 0+45	flow rate of t ++++++++++++++++++++++++++++++++++++	this hydrogr +++++++++++ 24 - H O U n o f f aph in 5 Q(CFS) 0 0.02 Q 0.03 Q 0.03 Q 0.03 Q 0.03 Q 0.03 Q 0.03 Q 0.05 Q 0.05 Q 0.05 Q 0.05 Q	aph = ++++++++++ R S T O H y d r o Minute int 2.5	4.230(CFS) +++++++++++ R M g r a p h cervals ((CFS))	
	Peak +++++ me(h+m) 0+ 5 0+10 0+15 0+20 0+25 0+20 0+25 0+30 0+35 0+40	flow rate of t ++++++++++++++++++++++++++++++++++++	this hydrogr ++++++++++ 24 - H O U n o f f 2000 Q 0.02 Q 0.03 Q 0.03 Q 0.03 Q 0.03 Q 0.03 Q 0.03 Q 0.03 Q 0.03 Q 0.05 Q 0.05 Q 0.05 Q 0.05 Q 0.05 Q 0.05 Q 0.05 Q 0.05 Q 0.05 Q	aph = ++++++++++ R S T O H y d r o Minute int 2.5	4.230(CFS) +++++++++++ R M g r a p h cervals ((CFS))	
	Peak +++++ me(h+m) 0+ 5 0+10 0+15 0+20 0+25 0+20 0+25 0+30 0+35 0+40 0+45	flow rate of t ++++++++++++++++++++++++++++++++++++	this hydrogr +++++++++++ 24 - H O U n o f f aph in 5 Q(CFS) 0 0.02 Q 0.03 Q 0.03 Q 0.03 Q 0.03 Q 0.03 Q 0.03 Q 0.05 Q 0.05 Q 0.05 Q 0.05 Q	aph = ++++++++++ R S T O H y d r o Minute int 2.5	4.230(CFS) +++++++++++ R M g r a p h cervals ((CFS))	
	Peak +++++ me(h+m) 0+ 5 0+10 0+15 0+20 0+25 0+20 0+25 0+30 0+35 0+40 0+45 0+50	flow rate of t ++++++++++++++++++++++++++++++++++++	this hydrogr ++++++++++ 24 - H O U n o f f 2000 Q 0.02 Q 0.03 Q 0.03 Q 0.03 Q 0.03 Q 0.03 Q 0.03 Q 0.03 Q 0.03 Q 0.05 Q 0.05 Q 0.05 Q 0.05 Q 0.05 Q 0.05 Q 0.05 Q 0.05 Q 0.05 Q	aph = ++++++++++ R S T O H y d r o Minute int 2.5	4.230(CFS) +++++++++++ R M g r a p h cervals ((CFS))	
	Peak +++++ me(h+m) 0+ 5 0+10 0+15 0+20 0+25 0+20 0+25 0+30 0+45 0+35 0+40 0+45 0+55 1+ 0	flow rate of t ++++++++++++++++++++++++++++++++++++	this hydrogr +++++++++++ 24 - H O U n o f f aph in 5 Q(CFS) 0 0.02 Q 0.03 Q 0.03 Q 0.03 Q 0.03 Q 0.03 Q 0.04 Q 0.05 Q 0.07 Q	aph = ++++++++++ R S T O H y d r o Minute int 2.5	4.230(CFS) +++++++++++ R M g r a p h cervals ((CFS))	
	Peak +++++ me(h+m) me(h+m) 0+5 0+10 0+15 0+20 0+25 0+20 0+25 0+30 0+25 0+30 0+35 0+40 0+45 0+55 1+ 0 1+ 5	flow rate of t ++++++++++++++++++++++++++++++++++++	this hydrogr +++++++++++ 24 - H O U n o f f Q(CFS) 0 0.02 Q 0.03 Q 0.03 Q 0.03 Q 0.03 Q 0.03 Q 0.03 Q 0.05 Q 0.06 Q 0.07 Q 0.06 Q	aph = ++++++++++ R S T O H y d r o Minute int 2.5	4.230(CFS) +++++++++++ R M g r a p h cervals ((CFS))	
	Peak +++++ me(h+m) 0+ 5 0+10 0+15 0+20 0+25 0+30 0+25 0+30 0+35 0+40 0+35 0+40 0+45 0+55 1+ 0 1+ 5 1+10	flow rate of t ++++++++++++++++++++++++++++++++++++	this hydrogr +++++++++++ 24 - H O U n o f f Q(CFS) 0 0.02 Q 0.03 Q 0.03 Q 0.03 Q 0.03 Q 0.03 Q 0.03 Q 0.05 Q 0.05 Q 0.05 Q 0.05 Q 0.05 Q 0.05 Q 0.07 Q 0.07 Q 0.07 Q 0.06 Q 0.05 Q	aph = ++++++++++ R S T O H y d r o Minute int 2.5	4.230(CFS) +++++++++++ R M g r a p h cervals ((CFS))	
	Peak +++++ me(h+m) 0+ 5 0+10 0+15 0+20 0+25 0+30 0+25 0+30 0+35 0+40 0+35 0+40 0+45 0+55 1+ 0 1+ 5 1+10 1+15	flow rate of t t+++++++++++++++++++++++++++++++++++	this hydrogr +++++++++++ 24 - H O U n o f f Q(CFS) 0 0.02 Q 0.03 Q 0.03 Q 0.03 Q 0.03 Q 0.03 Q 0.05 Q 0.05 Q 0.05 Q 0.05 Q 0.05 Q 0.05 Q 0.06 Q 0.05 Q 0.07 Q 0.05 Q	aph = ++++++++++ R S T O H y d r o Minute int 2.5	4.230(CFS) +++++++++++ R M g r a p h cervals ((CFS))	
	Peak +++++ me(h+m) 0+ 5 0+10 0+15 0+20 0+25 0+20 0+25 0+30 0+25 0+30 0+45 0+35 0+40 0+45 0+55 1+ 0 1+ 5 1+10 1+15 1+20	flow rate of t t+++++++++++++++++++++++++++++++++++	this hydrogr +++++++++++ 24 - H O U n o f f Q(CFS) 0 0.02 Q 0.03 Q 0.03 Q 0.03 Q 0.03 Q 0.03 Q 0.05 Q 0.05 Q 0.05 Q 0.05 Q 0.05 Q 0.05 Q 0.07 Q 0.07 Q 0.07 Q 0.07 Q 0.05 Q	aph = ++++++++++ R S T O H y d r o Minute int 2.5	4.230(CFS) +++++++++++ R M g r a p h cervals ((CFS))	
	Peak +++++ me(h+m) 0+ 5 0+10 0+15 0+20 0+25 0+20 0+25 0+30 0+45 0+35 0+40 0+45 0+45 0+55 1+0 1+ 5 1+10 1+5 1+20 1+25	flow rate of t ++++++++++++++++++++++++++++++++++++	this hydrogr +++++++++++ 24 - H O U n o f f Q(CFS) 0 0.02 Q 0.03 Q 0.03 Q 0.03 Q 0.03 Q 0.05 Q 0.05 Q 0.05 Q 0.05 Q 0.05 Q 0.05 Q 0.07 Q 0.07 Q 0.07 Q 0.07 Q 0.07 Q 0.07 Q 0.07 Q 0.05 Q	aph = ++++++++++ R S T O H y d r o Minute int 2.5	4.230(CFS) +++++++++++ R M g r a p h cervals ((CFS))	
	Peak +++++ me(h+m) 0+ 5 0+10 0+15 0+20 0+25 0+20 0+25 0+30 0+25 0+30 0+45 0+35 0+40 0+45 0+55 1+ 0 1+ 5 1+10 1+15 1+20	flow rate of t t+++++++++++++++++++++++++++++++++++	this hydrogr +++++++++++ 24 - H O U n o f f Q(CFS) 0 0.02 Q 0.03 Q 0.03 Q 0.03 Q 0.03 Q 0.03 Q 0.05 Q 0.05 Q 0.05 Q 0.05 Q 0.05 Q 0.05 Q 0.07 Q 0.07 Q 0.07 Q 0.07 Q 0.05 Q	aph = ++++++++++ R S T O H y d r o Minute int 2.5	4.230(CFS) +++++++++++ R M g r a p h cervals ((CFS))	

1+40	0.0069	0.05	Q	- B.	11	1	
1+45	0.0072	0.05	Q	1	1 I	1	
1+50	0.0076	0.06	Q	11	111	1	
1+55	0.0081	0.07	Q	10	1.0	1	
2+ 0	0.0086	0.07	Q	112	- Ťi	1	
2+ 5	0.0091	0.07	Q	i b	+ 11+	1	
2+10	0.0095	0.07	Q	10	i i i	1	
2+15	0.0100	0.07	Q	- 16	- 1 i i	1	
2+20	0.0105	0.07	Q	- ii	412	Ĵ.	
2+25	0.0110	0.07	Q	- miu	1 i l	í.	
2+30	0.0114	0.07	Q	- A	- i -	Ŭ.	
2+35	0.0120	0.08	Q	- ni	ť	1	
2+40	0.0125	0.08	Q	1	Ť.	i i	
2+45	0.0131	0.09	Q	i.	1	á i	
2+50	0.0137	0.09	Q	i.	i i		
2+55	0.0143	0.09	Q	È.	1	â.	
3+ 0	0.0149	0.09	Q	í.		1	
3+ 5	0.0155	0.09	Q	1	i	11	
3+10	0.0161	0.09	Q	i.		1.1	
3+15	0.0167	0.09	Q			9	
3+20	0.0173	0.09	Q			- 91	
3+25	0.0179	0.09	Q		1.1	1	
3+30	0.0185	0.09	Q	i	i i	- A -	
3+35	0.0191	0.09	Q	i -	i.	1 A A	
3+40	0.0197	0.09	Q	i i	1	i i	
3+45	0.0203	0.09	Q	i	8	1 a a	
3+50	0.0209	0.09	Q	i	1	1	
3+55	0.0216	0.10	Q	i	1.1	í.	
4+ 0	0.0223	0.10	Q	1	1	- 1 C	
4+ 5	0.0230	0.10	Q	Ú.	- A	i i	
4+10	0.0237	0.10	Q	1	1	- 1 C	
4+15	0.0245	0.10	Q	1	1	11	
4+20	0.0252	0.11	Q	1	1	1	
4+25	0.0260	0.12	Q	1	1	1	
4+30	0.0269	0.12	2	1	0.1	1.1	
4+35	0.0277	0.12	Q	1			
4+40	0.0285	0.12	Q	4	1	1	
4+45	0.0294	0.12	Q	de-	1	1	
4+50	0.0303	0.13	Q		1		
4+55	0.0312	0.14	Q	10	1)1	
5+ 0	0.0321	0.14	Q	0.0	-1	1	
5+ 5	0.0330	0.12	Q	1	1		
5+10	0.0337	0.11	Q		1	1	
5+15	0.0344	0.10	Q		1.	0	
5+20	0.0352	0.11	Q		1	1.	
5+25	0.0360	0.12	QV	- (t = .		10-1	
5+30	0.0369	0.12	QV	1	1	1	
5+35	0.0377	0.13	QV	1	Ţ	4	
5+40	0.0387	0.14	QV		1	0	
5+45	0.0396	0.14	QV	1	1)	
5+50	0.0406	0.14	QV	0		0	
5+55	0.0415	0.14	QV		1		
6+ 0	0.0425	0.14	QV	1	1		
6+ 5	0.0435	0.15	QV		1		

6+10	0.0445	0.15	QV		d, i	J.,	
6+15	0.0456	0.15	QV	1	1.1		
6+20	0.0467	0.16	QV	10	- (1)	1	
6+25	0.0477	0.16	QV		- (1)	1	
6+30	0.0488	0.16	QV	11	1	1	
6+35	0.0499	0.16	QV	HĎ -	0.0	d +	
6+40	0.0511	0.17	QV	1	Ť.		
6+45	0.0523	0.17	QV	1	- 1 f -	1	
6+50	0.0535	0.17	QV	1	1.1	Ĵ.	
6+55	0.0547	0.17	QV	i.	1	1	
7+ 0	0.0559	0.17	QV	j)	- 1b -	di-	
7+ 5	0.0571	0.17	QV	1	1	1	
7+10	0.0582	0.17	QV	1	1	1	
7+15	0.0594	0.17	QV		101	1	
7+20	0.0607	0.18	QV	i	10	1	
7+25	0.0620	0.19	QV	1		1	
7+30	0.0633	0.19	QV	1	1	1	
7+35	0.0646	0.20	QV	1	1	1	
7+40	0.0660	0.21	QV	Ĩ.	10	1	
7+45	0.0675	0.21	QV	Ú.	- D	1	
7+50	0.0689	0.21	QV	j.	4	- Te	
7+55	0.0705	0.22	QV	1		1	
8+ 0	0.0720	0.22	QV	1	- T	1	
8+ 5	0.0739	0.28	IQV	Û.	Ú.	1	
8+10	0.0762	0.33	IQV	1	i i	1	
8+15	0.0787	0.35	IQV	÷	- A	i i	
8+20	0.0812	0.37	IQV	1	1	1	
8+25	0.0838	0.38	IQV	- Air	1	1	
8+30	0.0865	0.39	IQV	i i	-1.	1	
8+35	0.0897	0.48	IQV	1	1	1	
8+40	0.0936	0.56	1 Q	3 C	1	- 10-1	
8+45	0.0976	0.58	IQ	1	1		
8+50	0.1023	0.68	10	1	1	1	
8+55	0.1075	0.76	IQ		1	- K. I	
9+ 0	0.1130	0.79	1 2	- 11	1	1	
9+ 5	0.1196	0.96	IQ	1	- H	1.1	
9+10	0.1273	1.12	I VQ)	- 1		
9+15	0.1352	1.16	I VQ	1	- A	1	
9+20	0.1439	1.26	I VQ	1	- f.	11	
9+25	0.1531	1.34	VQ I	1	- 1	1	
9+30	0.1626	1.37	I VQ	1	10	1111	
9+35	0.1726	1.46	I VQ	0	- U.	1	
9+40	0.1832	1.54	I VQ	+	-1	19.1	
9+45	0.1940	1.57	I VQ	1	1	- Q. I	
9+50	0.2055	1.66	I VQ	1		4	
9+55	0.2175	1.75	I Q	6	100 D	1	
10+ 0	0.2297	1.77	I VQ)	J.	0.	
10+5	0.2383	1.24	IQV	1	1	1	
10+10	0.2434	0.74	IQ V	1	1	14	
10+15	0.2478	0.64	IQ V	1	1	- 10 L	
10+20	0.2520	0.61	IQ V	() ()	- (h	1	
10+25	0.2562	0.62	IQ V	1	1		
10+30	0.2605	0.63	IQV	1	(1)		
10+35	0.2676	1.02	IQV		1		

10+40	0.2773	1.40	QVI	1 1
10+45	0.2875	1.49	QVI	
10+50	0.2980	1.53	QVI	
10+55	0.3086	1.54	Q V I	i i
11+ 0	0.3193	1.55	Q VI	i i
11+ 5	0.3294	1.48	Q VI	1
11+10	0.3392	1.41	Q VI	i i
11+15	0.3489	1.41	Q VI	1 1
11+20	0.3586	1.41	QV	1 1
11+25	0.3684	1.42	QV	A 11
11+30	0.3782	1.43	Q V	1
11+35	0.3871	1.28	QV	1 1
11+40	0.3950	1.15	Q IV	
11+45	0.4027	1.13		4. 1.
11+50	0.4110	1.20		4 4
11+55	0.4198	1.28	Q IV	4 D
12+ 0	0.4288	1.31	QIV	10.0
12+ 5	0.4416	1.86	QIV	
12+10	0.4581	2.39	QIV	
12+15	0.4753	2.50	Q V Q V Q V	
12+20	0.4935	2.63	Q V Q V	
12+25	0.5122	2.72	Q V Q V	1 6
12+30	0.5310	2.74		
12+35 12+40	0.5511 0.5722	2.91 3.07	IQ V IQ V	8 8
12+40	0.5936	3.10	IQ V	Y
12+50	0.6156	3.20	Q V	1 12
12+55	0.6383	3.29	V Q I	1 I I I I I I I I I I I I I I I I I I I
13+ 0	0.6611	3.31	Q V	4
13+ 5	0.6866	3.71	I Q V	7) Í
13+10	0.7148	4.09	i Q	V
13+15	0.7435	4.17	I Q	V
13+20	0.7726	4.21	I Q	V
13+25	0.8016	4.22	1 Q	I V I
13+30	0.8308	4.23	1 2	V
13+35	0.8541	3.38	IQ	V
13+40	0.8719	2.58	Q	1 V 1
13+45	0.8886	2,43	QI	I V I
13+50	0.9049	2.36	QI	V I
13+55	0.9212	2.37	QI	V I
14+ 0	0.9376	2.38	21	I V I
14+ 5	0.9562	2.70	2	
14+10	0,9769	3.00	1 Q	V I
14+15	0.9980	3.07 3.03		v i v i
14+20	1.0189	2.96		V VI
14+25 14+30	1.0392 1.0596	2.96		V
14+30	1.0799	2.95	10	v v
14+35	1.1003	2.95	IQ	l V
14+45	1.1208	2.90	10	I V
14+45	1.1408	2.90	IQ	i v
14+55	1.1603	2.83	IQ	I V
15+ 0	1.1798	2.83	IQ	V
15+ 5	1.1987	2.75	ÍQ	l v

15+10	1.2172	2.68	1			Q	to	i.	V 1
15+15	1.2356	2.68	1			Q	1	t	V
15+20	1.2536	2.60	1			Q		1	V I
15+25	1.2710	2.53	1			Q	10	1	V I
15+30	1.2884	2.53	1			Q		1	V 1
15+35	1.3037	2.22	1		Q	1.	- 1 1 - 1	1	V I
15+40	1,3170	1.93	1		Q)		1	V I
15+45	1.3299	1.88	1		Q	1	- 1 -	1	V I
15+50	1.3427	1.86			Q	-t	- L	1	V
15+55	1.3556	1.87	1		Q	1	14	1	V I
16+ 0	1.3685	1.87	1.		Q	-10	 () >	.1	V I
16+ 5	1.3758	1.06	1	Q		1	1	1	V I
16+10	1.3778	0.29	10			1	- (h)	1	VI
16+15	1.3788	0.14	Q			T.	1	(1)	VI
16+20	1.3793	0.07	Q			1	1		VI
16+25	1.3798	0.07	Q			15	10 A	1	VI
16+30	1.3802	0.07	Q			1			VI
16+35	1.3807	0.06	Q			1		1	VI
16+40	1.3810	0.05	Q			$ \mathbf{f} $		1	VI
16+45	1.3814	0.05	Q			4		1.1	VI
16+50	1.3817	0.05	Q			10		1	VI
16+55	1.3821	0.05	Q			4		1	VI
17+ 0	1.3825	0.05	Q			1			VI
17+ 5	1.3829	0.07	Q			1		- 15	VI
17+10	1.3835	0.08	Q			4	4		VI
17+15	1.3841	0.08	Q			15	1.2		VI
17+20	1.3847	0.09	Q			1	1		VI
17+25	1.3853	0.09	Q			1			VI
17+30	1.3859	0.09	Q			1		- 45	VI
17+35	1.3864	0.09	Q						VI
17+40	1.3870	0.09	Q						VI
17+45	1.3876	0.09	Q			4		1	VI
17+50	1.3882	0.08	Q			9			VI
17+55	1.3887	0.07	Q			1	1		VI
18+ 0	1.3891	0.07	Q			4		1	VI
18+ 5	1.3896	0.07	Q			22	1.1		
18+10	1.3901	0.07	Q						VI
18+15	1.3906	0.07	Q			1			VI
18+20	1.3911	0.07	Q					1	VI
18+25 18+30	1.3915 1.3920	0.07	2			1		- A	vi
18+30	1.3924	0.06	QQ					1	VI
18+40	1.3928	0.05	Q			È.		- i	VI
18+45	1.3932	0.05	Q			1		i.	VI
18+50	1.3935	0.04	Q			1.	1.12		VI
18+55	1.3937	0.04	Q			1.	1.2		VI
19+ 0	1.3940	0.04	Q			1		1.1	VI
19+ 5	1.3942	0.04	ĝ			i.	÷	i	VI
19+10	1.3946	0.05	ò			1	10.1	i.	VI
19+15	1.3949	0.05	Q			i		i.	VI
19+20	1.3954	0.06	õ			1		0.0	VI
19+25	1.3958	0.07	õ			Ú.		Ì	VI
19+30	1.3963	0.07	Q			1	1 C	Ì	VI
19+35	1.3967	0.06	Q			Ì.			VI
11.2. (1.2.)	20.003.00.00.0	1.00	12						

19+40	1.3971	0.05	Q	1	10	1	VI
19+45	1.3974	0.05	õ	1	i.	Ĵ.	V
19+50	1.3977	0.04	Q	12	Î.	1	VI
19+55	1.3980	0.04	õ	1	140	j.	VI
20+ 0	1.3982	0.04	Q	112	1 i i	- A	V
20+ 5	1.3985	0.04	Q	11	dia l	- i	VI
20+10	1.3989	0.05	Q			1	V
20+15	1.3992	0.05	Q		- (Î.)	1	VI
20+20	1.3996	0.05	Q	(1).	1	- il	VI
20+25	1.3999	0.05	õ		1 III	1	VI
20+30	1.4003	0.05	Q		1	di -	VI
20+35	1.4006	0.05	õ	11 C	l di l	Ĵ.	VI
20+40	1.4010	0.05	õ	1	i.	1	VI
20+45	1.4014	0.05	õ		- î	- i -	VI
20+50	1.4017	0.04	Q	i i	- Mini-	1	VI
20+55	1.4019	0.04	õ	1	i i i	Ŷ	VI
21+ 0	1.4022	0.04	Q	- ř		- Ý	VI
21+ 5	1.4024	0.04	Q	i.		- Î	VI
21+10	1.4028	0.05	Q	i i		i	VI
21+15	1.4031	0.05	Q	i.		1	VI
21+20	1.4034	0.04	Q			1.	VI
21+25	1.4037	0.04	Q		i i	i i	VI
21+30	1.4039	0.04	Q	í	- ń	í.	VI
21+35	1.4042	0.04	Q	- A	d l	i i	VI
21+40	1.4046	0.05	Q	- i	1	i i	VI
21+45	1.4049	0.05	Q	1 i	4	1	VI
21+50	1.4052	0.04	Q	i		1	VI
21+55	1.4055	0.04	Q	4	×	i	VI
22+ 0	1.4057	0.04	Q	- i	4	12	VI
22+ 5	1.4060	0.04	2	í	1		VI
22+10	1.4064	0.05	Q	- 3		1	VI
22+15	1.4067	0.05	Q				VI
22+20	1.4070	0.04	Q			1	VI
22+25	1.4073	0.04			10	1	VI
22+30	1.4075	0.04	Q		1		VI
22+35	1.4077	0.03	Q		i.	1	VI
22+40	1.4080	0.03					V
22+45	1.4082	0.03	Q		Ť.	i.	VI
22+50	1.4085	0.03	Q			0	VI
22+55	1.4087	0.03	Q	- G	Ť	- P	VI
23+ 0	1.4089	0.03	Q	1	4	6	VI
23+ 5	1.4092	0.03	Q		1		VI
23+10	1.4094	0.03	Q	1	i	10.0	VI
23+15	1.4096	0.03	Q	1.1	1	i i i	VI
23+20	1.4099	0.03	Q		1	- i i	VI
23+25	1.4101	0.03	Q	1		1	VI
23+30	1.4104	0.03	Q	1		î	VI
23+35	1.4104	0.03	Q	Ŷ	1	i.	VI
23+40	1.4108	0.03	Q	1		1	VI
23+45	1.4111	0.03	Q	÷			VI
23+45	1.4111	0.03	Q				VI
23+50	1.4116	0.03	Q	12	S 2		v
24+ 0	1.4118	0.03	Q	1	14	10 C	VI
24+ 5	1.4119	0.02	Q				vi
271 3	T. 47.7.2	0.04	8	÷.			

+10	1.4119	0.00	Q			VI
+15	1.4120	0.00	Q	T.	 	VI

APPENDIX D

ON-SITE HYDROLOGY BASED ON PROPOSED CONDITION (UNIT HYDROGRAPH)

APPENDIX D.1

100-YEAR UNIT HYDROGRAPH CALCULATIONS (PROPOSED)

Unit Hydrograph Analysis Copyright (c) CIVILCADD/CIVILDESIGN, 1989 - 2004, Version 7.0 Study date 05/13/21 File: 14820001POUN24100.out Riverside County Synthetic Unit Hydrology Method RCFC & WCD Manual date - April 1978 Program License Serial Number 4042 English (in-lb) Input Units Used English Rainfall Data (Inches) Input Values Used English Units used in output format POST-DEVELOPMENT APN 316-211-014 - PROPOSED TRUCK/TRAILER PARKING/STORAGE LOT 100-YEAR STORM EVENT Drainage Area = 8.88(Ac.) = 0.014 Sq. Mi. Drainage Area for Depth-Area Areal Adjustment = 8.88(Ac.) = 0.014 Sq. Mi. Length along longest watercourse = 850.00(Ft.) Length along longest watercourse measured to centroid = 148.00(Ft.) Length along longest watercourse = 0.161 Mi. Length along longest watercourse measured to centroid = 0.028 Mi. Difference in elevation = 8.30(Ft.) Slope along watercourse = 51.5576 Ft./Mi. Average Manning's 'N' = 0.015 Lag time = 0.022 Hr. Lag time = 1.31 Min. 25% of lag time = 0.33 Min. 40% of lag time = 0.52 Min. Unit time = 5.00 Min. Duration of storm = 24 Hour(s) User Entered Base Flow = 0.00(CFS) 2 YEAR Area rainfall data: Weighting[1*2] Area(Ac.)[1] Rainfall(In)[2] 8.88 1.89 16.78 100 YEAR Area rainfall data: Area(Ac.)[1] Rainfall(In)[2] Weighting[1*2] 42.80 8.88 4.82

STORM EVENT (YEAR) = 100.00 Area Averaged 2-Year Rainfall = 1.890(In) Area Averaged 100-Year Rainfall = 4.820(In) Point rain (area averaged) = 4.820(In) Areal adjustment factor = 100.00 % Adjusted average point rain = 4.820(In) Sub-Area Data: Area(Ac.) Runoff Index Impervious % 8.06098.001.0000.82056.000.000 Total Area Entered = 8.88(Ac.) RI RI Infil. Rate Impervious Adj. Infil. Rate Area% F

 AMC2
 AMC-3
 (In/Hr)
 (Dec.%)
 (In/Hr)
 (Dec.)
 (In/Hr)

 98.0
 99.2
 0.010
 1.000
 0.001
 0.908
 0.001

 56.0
 74.8
 0.305
 0.000
 0.305
 0.092
 0.028

 Sum (F)
 =
 0.029

 Area averaged mean soil loss (F) (In/Hr) = 0.029 Minimum soil loss rate ((In/Hr)) = 0.015 (for 24 hour storm duration) Soil low loss rate (decimal) = 0.900 Unit Hydrograph VALLEY S-Curve _____ Unit Hydrograph Data Unit time period Time % of lag Distribution Unit Hydrograph (hrs) Graph % (CFS) Unit Time
(Hr.)Pattern
(In/Hr)Storm Rain
(In/Hr)Loss rate (In./Hr)
Max
IEffective
(In/Hr)10.080.070.0390.0520.0350.0020.170.070.0390.0510.0350.0030.250.070.0390.0510.0350.0040.330.100.0580.051----0.0150.420.100.0580.051----0.0160.500.100.0580.050----0.0170.580.100.0580.050----0.0180.670.100.0580.050----0.0190.750.100.0580.050----0.01100.830.130.0770.050---0.03110.920.130.0770.049---0.03131.080.100.0580.049---0.01141.170.100.0580.049---0.01161.330.100.0580.049---0.01171.420.100.0580.049---0.01

	18	1.50	0.10	0.058	0.048		0.01
	19	1.58	0.10	0.058	0.048		0.01
	20	1.67		0.058	0.048		0.01
	21	1.75		0.058	0.048		0.01
	22	1.83		0.077	0.048	444	0.03
	23	1.92	0.13	0.077	0.047	111	0.03
	24	2.00		0.077	0.047		0.03
	25	2.08	0.13	0.077	0.047		0.03
	26	2.17		0.077	0.047	6.1.1.1.	0.03
	27	2.25	0.13	0.077	0.047	2.2.2	0.03
						111	
	28	2.33	0.13	0.077	0.046		0.03
	29	2.42	0.13	0.077	0.046		0.03
	30	2.50	0.13	0.077	0.046	(2.2.2)	0.03
	31	2.58		0.096	0.046		0.05
	32	2.67	0.17	0.000	0.046	1997 - C	0.05
	33		0.17	0.096	0.045		0.05
	34	2.83	0.17	8 * 5 ° ° °	0.045	****	0.05
1	35	2.92	0.17	0.096	0.045		0.05
2	36	3.00	0.17	0.096	0.045	385	0.05
1	37	3.08	0.17	0.096	0.045		0.05
3	38	3.17	0.17	0.096	0.045		0.05
3	39	3.25	0.17	0.096	0.044		0.05
	40	3.33		0.096	0.044		0.05
	41	3.42	0.17		0.044		0.05
	42	3.50	0.17		0.044		0.05
	43	3.58	0.17	0.096	0.044		0.05
	44	3.67	0.17	0.096	0.043		0.05
	45	3.75	0.17	0.096	0.043	· 613	0.05
		3.83					
	46		0.20	0.116	0.043		0.07
	17	3.92	0.20	0.116	0.043	777	0.07
	48	4.00	0.20	0.116	0.043		0.07
	19	4.08	0.20	0.116	0.043		0.07
	50	4.17	0.20	0.116	0.042		0.07
	51	4.25	0.20	0.116	0.042		0.07
	52	4.33	0.23	0.135	0.042		0.09
	53	4.42	0.23	0.135	0.042		0.09
5	54	4.50	0.23	0.135	0.042		0.09
1	55	4.58	0.23	0.135	0.041	777	0.09
5	56	4.67	0.23	0.135	0.041		0.09
5	57	4.75	0.23	0.135	0.041		0.09
5	58	4.83	0.27	0.154	0.041		0.11
5	59	4.92	0.27	0.154	0.041		0.11
	50	5.00	0.27	0.154	0.041		0.11
	51	5.08	0.20	0.116	0.040		0.08
		5.17	0.20	0.116	0.040		0.08
		5.25	0.20	0.116	0.040		0.08
	54	5.33	0.23	0.135	0.040		0.10
		5.42	0.23	0.135	0.040		0.10
		5.50	0.23	0.135	0.040		0.10
	56						
		5.58	0.27	0.154	0.039		0.11
	58	5.67	0.27	0.154	0.039		0.12
	59	5.75	0.27	0.154	0.039		0.12
	70	5.83	0.27	0.154	0.039		0.12
	71	5.92	0.27	0.154	0.039		0.12
		6.00	0.27	0.154	0.038		0.12
		6.08		0.174	0.038		0.14
		6.17		0.174	0.038		0.14
		6.25	0.30	0.174	0.038		0.14
5	76	6.33	0.30	0.174	0.038	200	0.14
17	77	6.42	0.30	0.174	0.038		0.14

78	6.50	0.30	0.174	0.037		0.14
79	6.58	0.33	0.193	0.037		0.16
80	6.67	0.33	0.193	0.037	6.10	0.16
81	6.75	0.33	0.193	0.037		0.16
82	6.83	0.33	0.193	0.037	-222	0.16
83	6.92	0.33	0.193	0.037	222	0.16
84	7.00	0.33	0.193	0.036	7.121	0.16
85	7.08	0.33	0.193	0.036	177	0.16
86	7.17	0.33	0.193	0.036	577	0.16
87	7.25	0.33	0.193	0.036		0.16
88	7.33	0.37	0.212	0.036		0.18
89	7.42	0.37	0.212	0,036		0.18
90	7.50	0.37	0.212	0.035		0.18
91	7.58	0.40	0.231	0.035	7.77	0.20
92	7.67	0.40	0.231	0.035		0.20
93	7.75	0.40	0.231	0.035		0.20
94	7.83	0.43	0.251	0.035		0.22
95	7.92	0.43	0.251	0,035		0.22
96	8.00	0.43	0.251	0.034		0.22
97	8.08	0.50	0.289	0.034		0.25
98	8.17	0.50	0.289	0.034		0.26
99	8.25	0.50	0.289	0.034	تعريد بلا	0.26
100	8.33	0.50	0.289	0.034		0.26
101	8.42	0.50	0.289	0.034		0.26
102	8.50	0.50	0.289	0.034		0.26
103	8.58	0.53	0.308	0.033		0.28
104	8.67	0.53	0.308	0.033		0.28
105	8.75	0.53	0.308	0.033	-	0.28
106	8.83	0.57	0.328	0.033		0.29
107	8.92	0.57	0.328	0.033		0.30
108	9.00	0.57	0.328	0.033	220	0.30
109	9.08	0.63	0.366	0.032		0.33
110	9.17	0.63	0.366	0.032	122	0.33
			0.366			0.33
111	9.25	0.63		0.032	100	
112	9.33	0.67	0.386	0.032	528	0.35
113	9.42	0.67	0.386	0.032		0.35
114	9.50	0.67	0.386	0.032		0.35
115	9.58	0.70	0.405	0.032		0.37
116	9.67	0.70	0.405	0.031		0.37
117	9.75	0.70	0.405	0.031		0.37
118	9.83	0.73	0.424	0.031		0.39
119	9.92	0.73	0.424	0.031		0.39
120	10.00	0.73	0.424	0.031		0.39
121	10.08	0.50	0.289	0.031		0.26
122	10.17	0.50	0.289	0.030		0.26
123	10.25	0.50	0.289	0.030		0.26
124	10.33	0.50	0.289	0.030		0.26
125	10.42	0.50	0.289	0.030		0,26
126	10.50	0.50	0.289	0.030		0.26
127	10.58	0.67	0.386	0.030		0.36
128	10.67	0.67	0.386	0.030		0.36
129	10.75	0.67	0.386	0.029		0.36
130	10.83	0.67	0.386	0.029		0.36
131	10.92	0.67	0.386	0.029		0.36
132	11.00	0.67	0.386	0.029		0.36
133	11.08	0.63	0.366	0.029		0.34
134	11.17	0.63	0.366	0.029		0.34
135	11.25	0.63	0.366	0.029		0.34
136	11.33	0.63	0.366	0.028		0.34
137	11.42	0.63	0.366	0.028		0.34
101	11.12	9.00	0.000	0.000		0.01

138	11.50	0.63	0.366	0.028		0.34
139	11.58	0.57	0.328	0.028	++++	0.30
140	11.67	0.57	0.328	0.028		0.30
141	11.75	0.57	0.328	0.028		0.30
142	11.83	0,60	0.347	0.028		0.32
143	11.92	0.60	0.347	0.027		0.32
144	12.00	0.60	0.347	0.027		0.32
145	12.08	0.83	0.482	0.027		0.45
146	12,17	0.83	0.482	0.027		0.45
147	12.25	0.83	0.482	0.027		0.46
148	12.33	0.87	0.501	0.027		0.47
149	12.42	0.87	0.501	0.027	111	0.47
150	12.50	0.87	0.501 0.540	0.027	222	0.51
151 152	12.58 12.67	0.93	0.540	0.026	622	0.51
153	12.75	0.93	0.540	0.026		0.5
154	12.83	0.97	0.559	0.026		0.53
155	12.92	0.97	0.559	0.026	111	0.53
156	13.00	0.97	0.559	0.026		0.53
157	13.08	1.13	0.656	0.026	تعتبانيا	0.63
158	13.17	1.13	0.656	0.025		0.63
159	13.25	1.13	0.656	0.025		0.63
160	13.33	1.13	0.656	0.025		0.63
161	13.42	1.13	0.656	0.025		0.63
162	13.50	1.13	0.656	0.025		0.6
163	13.58	0.77	0.443	0.025		0.42
164	13.67	0.77	0.443	0.025		0.42
165	13.75	0.77	0.443	0.025		0.4
166	13.83	0.77	0.443	0.024		0.4
167	13.92	0.77	0.443	0.024	and the second	0.42
168	14.00	0.77	0.443	0.024 0.024		0.42
169 170	14.08 14.17	0.90	0.521 0.521	0.024		0.5
	14.17	0.90	0.521	0.024	822	0.5
171 172	14.33	0.87	0.501	0.024		0.4
173		0.87	0.501	0.024	ليهي	0.4
174		0.87	0.501	0.023		0.4
175	14.58	0.87	0.501	0.023	444	0.4
176	14.67	0.87	0.501	0.023		0.4
177	14.75	0.87	0.501	0.023		0.4
178	14.83	0.83	0.482	0.023		0.4
179	14.92	0.83	0.482	0.023		0.4
180	15.00	0.83	0.482	0.023		0.4
181	15.08	0.80	0.463	0.023		0.4
182	15.17	0.80	0.463	0.023		0.4
183	15.25	0.80	0.463	0.022		0.4
184	15.33	0.77	0.443	0.022		0.4
185	15.42	0.77	0.443	0.022		0.4
186	15.50	0.77	0.443	0.022		0.4
187	15.58	0.63	0.366	0.022		0.3
188	15.67	0.63	0.366	0.022	232	0.3
189	15.75	0.63	0.366 0.366	0.022		0.3
190 191	15.83 15.92	0.63	0.366	0.022	222	0.3
191	16.00	0.63	0.366	0.021	122	0.3
192	16.00	0.13	0.077	0.021		0.0
194	16.17	0.13	0.077	0.021		0.0
195	16.25	0.13	0.077	0.021		0.0
196	16.33	0.13	0.077	0.021		0.0
197	16.42	0.13	0.077	0.021		0.0

1.00	10.00	0.10	0.077	0 001		0.06
198	16.50	0.13	0.077	0.021		0.06
199	16.58	0.10	0.058	0.021		0.04
200	16.67	0.10	0.058	0.021	444	0.04
201	16.75	0.10	0.058	0.020	177	0.04
202	16.83	0.10	0.058	0.020		0.04
203	16.92	0.10	0.058	0.020		0.04
204	17.00	0.10	0.058	0.020		0.04
205	17.08	0.17	0.096	0.020		0.08
206	17.17	0.17	0.096	0.020		0.08
207	17.25	0.17	0.096	0.020		0.08
208	17.33	0.17	0.096	0.020		0.08
209	17.42	0.17	0.096	0.020		0.08
210	17.50	0.17	0.096	0.020		0.08
211	17.58	0.17	0.096	0.019		0.08
212	17,67	0.17	0.096	0.019		0.08
213	17.75	0.17	0.096	0.019		0.08
214	17.83	0.13	0.077	0.019		0.06
215	17.92	0.13	0.077	0.019		0.06
216	18.00	0.13	0.077	0.019		0.06
217	18.08	0.13	0.077	0.019	622	0.06
	18.17	0.13	0.077	0.019	000	0.06
218				0.019	1992	0.06
219	18.25	0.13	0.077		1.4.4	0.06
220	18.33	0.13	0.077	0.019		
221	18.42	0.13	0.077	0.018		0.06
222	18.50	0.13	0.077	0.018		0.06
223	18.58	0.10	0.058	0.018		0.04
224	18.67	0.10	0.058	0.018	1000	0.04
225	18.75	0.10	0.058	0.018	202	0.04
226	18.83	0.07	0.039	0.018		0.02
227	18,92	0.07	0.039	0.018		0.02
228	19.00	0.07	0.039	0.018		0.02
229	19.08	0.10	0.058	0.018		0.04
230	19.17	0.10	0.058	0.018		0.04
231	19.25	0.10	0,058	0.018		0.04
232	19.33	0.13	0.077	0.018	the second s	0.06
233	19.42	0.13	0.077	0.017		0.06
234	19.50	0.13	0.077	0.017		0.06
235	19.58	0.10	0.058	0.017	maa	0.04
236	19.67	0.10	0.058	0.017		0.04
237	19.75	0.10	0.058	0.017		0.04
238	19.83	0.07	0.039	0.017		0.02
239	19.92	0.07	0.039	0.017	to the feet free	0.02
240	20.00	0.07	0.039	0.017		0.02
241	20.08	0.10	0.058	0.017		0.04
242	20.17	0.10	0.058	0.017		0.04
243	20.25	0.10	0.058	0.017		0.04
244	20.33	0.10	0.058	0.017		0.04
245	20.33	0.10	0.058	0.017		0.04
245	20.50	0.10	0.058	0.016		0.04
		0.10	0.058	0.016	-	0.04
247	20.58			0.016		0.04
248	20.67	0.10	0.058			0.04
249	20.75	0.10	0.058	0.016	1000	
250	20.83	0.07	0.039	0.016		0.02
251	20.92	0.07	0.039	0.016		0.02
252	21.00	0.07	0.039	0.016		0.02
253	21.08	0.10	0.058	0.016		0.04
254	21.17	0.10	0.058	0.016	1.000	0.04
255	21.25	0.10	0.058	0.016		0.04
256	21.33	0.07	0.039	0.016		0.02
257	21.42	0.07	0,039	0.016		0.02
		(1777) (1777)				

258							
	21.50	0.07	0.039	0,016		0.02	
259	21.58	0.10	0.058	0.016		0.04	
260	21.67	0.10	0.058	0.016		0.04	
261	21.75	0.10	0.058	0.016		0.04	
262	21.83	0.07	0.039	0.015		0.02	
263	21.92	0.07	0.039	0.015		0.02	
64	22.00		0.039	0.015		0.02	
265	22.08	0.10	0.058	0.015		0.04	
66	22.00		0.058	0.015		0.04	
		0.10	0.058	0.015	<u></u>	0.04	
67	22.25				110		
68	22.33	0.07	0.039	0.015		0.02	
69	22.42	0.07	0.039	0.015		0.02	
70	22.50	0.07	0.039	0.015	+==	0.02	
71	22.58	0.07	0.039	0.015	1942	0.02	
72	22.67	0.07	0.039	0.015		0.02	
73	22.75	0.07	0.039	0.015		0.02	
74	22.83	0.07	0.039	0.015		0.02	
75	22.92	0.07	0.039	0.015		0.02	
76	23.00	0.07	0.039	0.015		0.02	
77	23.08	0.07	0.039	0.015		0.02	
78	23.17	0.07	0.039	0.015		0.02	
79	23.25	0.07	0.039	0.015		0.02	
80	23.33	0.07	0.039	0.015		0.02	
81	23.42	0.07	0.039	0.015		0.02	
				0.015	122	0.02	
82	23.50	0.07	0.039				
83	23.58	0.07	0.039	0.015		0.02	
84	23.67	0.07	0.039	0.015	100	0.02	
85	23.75	0.07	0.039	0.015		0.02	
86	23.83	0.07	0.039	0.015		0.02	
287	23.92	0.07	0.039	0.015		0.02	
88	24.00	0.07	0.039	0.015	<u>محد</u>	0.02	
	Total Total Total Flood	soil loss = soil loss = rainfall = volume =	0.70 0.515 4.82(132947.	(Ac.Ft) In) 3 Cubic Feet	3.1(Ac	.Ft)	
	Total	soil loss =	224	19.9 Cubic Feet			
	Peak	flow rate of	this hyd	rograph =	5.645(CFS)		
	the bar best the set of		and have been been and the been been	en ent bies bes net ent des net ent bes her her auf auf			
	+++++++	+++++++++++++++++++++++++++++++++++++++	++++++++	+++++++++++++++++++++++++++++++++++++++	+++++++++++++++++++++++++++++++++++++++	+++++++++++	de de se se
	 +++++++	++++++++++++	+++++++ 24 - H	++++++++++++++++++++++++++++++++++++++	++++++++++++ R M	*****	* * *
	++++++		24 - H	++++++++++++++++++++++++++++++++++++++	RM	******	****
	+++++	Rü	24 - H n o f f	ОИК STO Нудго	RM graph		++++
	+++++	Rü	24 - H n o f f	OUR STO	RM graph		++++
Tim		R u Hydrogr	24 - H n o f f aph in	OUR STO Hydro	R M g r a p h ervals ((CFS)) 	
	ie (h+m)	R u Hydrogr Volume Ac.Ft	24 - H n o f f aph in Q(CFS)	OUR STO Hydro 5 Minute int 0 2.5	R M g r a p h ervals ((CFS)) 	
	ue (h+m)	R u Hydrogr Volume Ac.Ft 0.0002	24 - H n o f f aph in Q(CFS) 0.02	OUR STO Hydro 5 Minute int 0 2.5 Q	R M g r a p h ervals ((CFS)) 	
 0 0	ne (h+m) 	R u Hydrogr Volume Ac.Ft 0.0002 0.0004	24 - H n o f f aph in Q(CFS) 0.02 0.03	OUR STO Hydro 5 Minute int 0 2.5 Q Q	R M g r a p h ervals ((CFS)) 	
 0 0 0	ne (h+m))+ 5)+10)+15	R u Hydrogr Volume Ac.Ft 0.0002 0.0004 0.0006	24 - H n o f f aph in Q(CFS) 0.02 0.03 0.03	OUR STO Hydro 5 Minute int 0 2.5 Q Q Q	R M g r a p h ervals ((CFS)) 	
0 0 0 0	ne(h+m) 0+ 5 0+10 0+15 0+20	R u Hydrogr Volume Ac.Ft 0.0002 0.0004 0.0006 0.0010	24 - H n o f f aph in Q(CFS) 0.02 0.03 0.03 0.03 0.05	OUR STO Hydro 5 Minute int 0 2.5 Q Q Q Q Q	R M g r a p h ervals ((CFS)) 	
0 0 0 0 0	ue (h+m))+ 5)+10)+15)+20)+25	R u Hydrogr Volume Ac.Ft 0.0002 0.0004 0.0006 0.0010 0.0014	24 - H n o f f aph in Q(CFS) 0.02 0.03 0.03 0.03 0.05 0.06	OUR STO Hydro 5 Minute int 0 2.5 Q Q Q Q Q Q Q Q	R M g r a p h ervals ((CFS)) 	
000000000000000000000000000000000000000	ne (h+m))+ 5)+10)+15)+20)+25)+30	R u Hydrogr Volume Ac.Ft 0.0002 0.0004 0.0006 0.0010 0.0014 0.0018	24 - H n o f f aph in Q(CFS) 0.02 0.03 0.03 0.05 0.06 0.06	OUR STO Hydro 5 Minute int 0 2.5 Q Q Q Q Q Q Q Q Q Q	R M g r a p h ervals ((CFS)) 	
	ne (h+m) + 5 +10 +15 +20 +25 +30 +35	R u Hydrogr Volume Ac.Ft 0.0002 0.0004 0.0006 0.0010 0.0014 0.0018 0.0023	24 - H n o f f aph in Q(CFS) 0.02 0.03 0.03 0.03 0.05 0.06 0.06 0.07	OUR STO Hydro 5 Minute int 0 2.5 Q 1 Q 1 Q 1 Q 1 Q 1 Q 1 Q 1 Q 1 Q 1 Q 1	R M g r a p h ervals ((CFS)) 	 10.
	ne (h+m))+ 5)+10)+15)+20)+25)+30	R u Hydrogr Volume Ac.Ft 0.0002 0.0004 0.0006 0.0010 0.0014 0.0018	24 - H n o f f aph in Q(CFS) 0.02 0.03 0.03 0.03 0.05 0.06 0.06 0.07 0.07	OUR STO Hydro 5 Minute int 0 2.5 Q Q Q Q Q Q Q Q Q Q	R M g r a p h ervals ((CFS)) 	

)+50	0.0045	0.18	Q	n i	11	Î	Î
)+55	0.0061	0.24	Q	Ú.			1
L+ 0	0,0078	0.25	Q		1	1	1
L+ 5	0.0088	0.14	Q	1	- 1	1	1
L+10	0.0093	0.08	Q	1	1	1	1
L+15	0.0099	0.08	Q	1			1
L+20	0.0104	0.08	Q	1	- 1	1+	1
1+25	0,0110	0.08	Q	1		.0	1
1+30	0.0116	0.08	Q	1	1	1	
L+35	0.0122	0.09	Q	1	1.	1	- 1
L+40	0.0128	0.09	Q		1	1)
L+45	0.0134	0.09	Q	1	1	4.24	
L+50	0.0148	0.20	Q	1	- L.	0.0	
L+55	0,0166	0,26	VQ	1	1	0	- 30
2+ 0	0.0184	0.27	VQ		1		1
2+ 5	0.0203	0.27	VQ		1		- 18
2+10	0.0222	0.27	VQ	9 -	1	C	- 19
2+15	0.0240	0.27	VQ	1	1		
2+20	0.0259	0.27	VQ	3 -	1		
2+25	0.0278	0.28	VQ	1.1	1		
2+30	0.0297	0.28	VQ		Į.		
2+35	0.0324	0.39	VQ		Ţ.		- 0
2+40	0.0355	0.45	VQ	1			1
2+45	0.0386	0.46	VQ				
2+50	0.0418	0.46	VQ	1. I	1		
2+55	0.0449	0.46	VQ				4
3+ 0	0.0481	0.46	VQ		- ÷	1	
3+ 5	0.0513	0.46	VQ	12	- 44		
3+10 3+15	0.0545 0.0577	0.46 0.47	VQ VQ	12	1		
3+10	0.0609	0.47	VQ		1		
3+25	0.0641	0.47	VQ	12		1	×.
3+30	0.0674	0.47	VQ		1		ř
3+35	0.0706	0.47	VQ	1.1	÷.	i.	i i
3+40	0.0739	0.47	VQ			î	i.
3+45	0.0772	0.48	IQ	- i.	- i -	i.	i
3+50	0.0812	0.59	IVQ	- P	1	-1	Ú.
3+55	0.0857	0.65	IVQ	i i		i.	i.
4+ 0	0.0902	0.65	IVQ	- io	1	1	(i)
4+ 5	0.0947	0.65	IVQ	1		Î.	Ú.
4+10	0.0992	0.66	VQ	i i	1	1 i	Ť.
4+15	0,1037	0.66	IVQ	1			Ú.
4+20	0.1090	0.77	IV Q	1	1		
4+25	0.1147	0.83	IV Q	1	1	1	1
4+30	0.1205	0.84	IV Q	1		1	1
4+35	0.1262	0.84	IV Q	1		1	1
4+40	0.1320	0.84	V Q	1	1	1	1
4+45	0.1378	0.84	IV Q	1		1	1
4+50	0.1443	0.95	IV Q	1	1	1	1
4+55	0.1513	1.01	IV Q				1
5+ 0	0,1583	1.02	V Q			1	1
5+ 5	0.1638	0.80	I VQ	1			9
5+10	0.1685	0.69	IQ)		1	1
5+15	0.1732	0.68	10	10	1	1	4
5+20	0.1786	0.79	I VQ	1	1		
5+25	0,1845	0.85	VQ I		1		1
5+30	0.1903	0.85	I VQ	1	1	- 1	1
5+35	0.1970	0.97	I VQ		1	1	1
5+40	0.2040	1,02	VQ	P	- E	- 0	1
5+45	0.2112	1.03	IVQ		1		

5+50	0.2183	1.03	IVQ I I	n l
5+55	0.2254	1.03	VQ	
6+ 0	0.2325	1.04	VQ	1
6+ 5	0.2404	1.15	I VQ I I	de la
6+10	0.2487	1.21	VQ I I	1
6+15	0.2571	1.21	VQ I I	1
6+20	0.2655	1.21	VQ I I	1
6+25	0.2738	1.22	VQ I	- I-
6+30	0.2822	1.22	I VQ I I	1
6+35	0.2914	1.33	IVQIII	1
6+40	0.3009	1.39	I VQ I I	1
6+45	0.3105	1.39	I VQ I I	1
6+50	0.3202	1.40	I VQ I	1
6+55	0.3298	1.40	VQ I	1
7+ 0	0.3394	1.40	I VQ I I	1
7+ 5	0.3491	1.40	VQ I	1
7+10	0.3587	1.40	VQ I	1
7+15	0.3684	1.40	VQ	ų:
7+20	0.3788	1.52	VQI	1
7+25	0.3897	1.57	VQ I	-
7+30	0.4006	1.58		<u>k.</u>
7+35	0.4122	1.69	VQ I	V
7+40	0.4243	1.75		
7+45	0.4364	1.76		
7+50	0.4493	1.87		1
7+55	0.4625	1.93	VQ VQ	1
8+ 0 8+ 5	0.4759 0.4907	1.93 2.16	VQ I	1
8+10	0.5064	2.27	V QI	8.
8+15	0.5221	2.28	V QI	
8+20	0.5378	2.29	V QI I	
8+25	0.5536	2.29	V QI I	1
8+30	0.5693	2.29	V QI I	
8+35	0.5859	2.40	i vậi i	1.
8+40	0.6028	2.46	I V QI I	i i
8+45	0.6198	2.47	VQI	ii ii
8+50	0.6375	2.58	I VQ I	-j-
8+55	0.6557	2.64	V Q I	1
9+ 0	0.6739	2.64	V Q I	1
9+ 5	0.6936	2.86	VIQ I	1
9+10	0.7141	2.98	V Q I	1
9+15	0.7347	2.99	I QIV	1
9+20	0.7561	3.10	VIQ I	T.
9+25	0.7779	3.16	V Q I	
9+30	0.7997	3.17	I VQ I	1
9+35	0.8223	3.28	I VQ I	1
9+40	0.8453	3.34	I IVQ I	1
9+45	0.8683	3.35	VQ	1
9+50	0.8921	3.46	I IVQ I	1
9+55	0.9163	3.51	IVQI	1
10+ 0	0.9406	3.52	IVQ	
10+ 5	0.9595	2.75	I IQV I	3
10+10	0.9757	2.36	I QIV I	1
10+15	0.9917	2.32		
10+20	1.0077	2.32		3
10+25	1,0237	2.32	QI V I	
10+30	1.0396	2.32	Q V I	3
10+35	1.0594	2.87		0
10+40	1.0812	3.16		
10+45	1.1031	3.19		

0+50 0+55	1.1251 1.1471	3.19 3.19	
1+ 0	1.1691	3.19	
1+ 5	1,1903	3.08	QVI
1+10	1.2112	3.03	IQVI
1+15	1,2320	3,02	IQVIII
1+20	1.2528	3.02	IQVI
1+25	1.2737	3.03	IQVI
1+30	1.2945	3.03	Q V I
1+35	1.3138	2.81	10 V
1+40	1,3324	2.70	
1+45 1+50	1.3509 1.3702	2.69	
1+55	1.3898	2.86	
2+ 0	1.4095	2.86	
2+ 5	1.4346	3.63	QVIII
2+10	1.4624	4.03	I Q VI I
2+15	1.4904	4.07	1 Q VI I
2+20	1.5192	4.19	I Q VI I
2+25	1.5485	4.24	IQVI
2+30	1.5777	4.25	I Q V I
2+35	1.6085	4.47	Q V
2+40	1.6401	4.59	
2+45 2+50	1.6718 1.7042	4.60 4.71	
2+55	1.7371	4.77	
3+ 0	1.7700	4.78	
3+ 5	1.8067	5.33	I IQV I
3+10	1.8453	5.61	IQVI
3+15	1.8842	5.64	IQVI
3+20	1,9230	5.64	I Q V I
3+25	1.9619	5.64	I Q V I
3+30	2.0008	5.65 1	
3+35 3+40	2.0313	4.44	
3+40	2.0576 2.0834	3.81 3.75	
3+50	2,1092	3.75	
3+55	2.1351	3.75	
4+ 0	2.1609	3.75	
4+ 5	2.1898	4.19	Q V V
4+10	2.2203	4.42	Q VI
4+15	2.2509	4.45	I Q I VI
4+20	2.2808	4.34	I Q I VI
4+25	2.3103	4.28	
4+30 4+35	2.3397 2.3692	4.28 4.28	Q V Q V Q V Q V Q IV Q IV
4+40	2.3987	4.28	
4+45	2.4282	4.28	Q I IV
4+50	2.4569	4.17	
4+55	2.4853	4.12	
5+ 0	2.5136	4.11	Q V
5+ 5	2.5411	4.00	IQIIV
5+10	2.5683	3,95	I Q I I V
5+15	2.5955	3.94 1	Q V Q V Q V Q V
5+20	2.6219	3.83	
5+25	2.6479	3.78	
5+30	2.6739	3.77	
5+35 5+40	2.6968 2.7182	3.33 3.11	
5+45	2.7395	3.08	
	~	0.00	1 xe

15+50	2.7607	3.09	1	1 Q		V
15+55	2.7820	3.09	1	I Q		1 V
16+ 0	2.8032	3.09	1 .	IQ	1	I V
16+ 5	2.8131	1.44	1 Q	1	1	I V
16+10	2.8172	0.58	10	1	1	I V
16+15	2.8206	0.50	1 0	1	1	I V
16+20	2.8241	0.50	10	Ú.	1	I V
16+25	2.8275	0.50	ÌQ	í	1	V
16+30	2.8310	0.50	ÌQ	i i	÷	V
16+35	2.8337	0.40	Q	1	1	i v
16+40	2.8361	0.34	10		1	v
16+45	2.8384	0.33	10	4		v
		0.33		1		v
16+50	2.8407		10	1	1	v
16+55	2.8430	0.34	10	4	1	
17+ 0	2.8453	0.34	10	1	1	V
17+ 5	2.8492	0.56	1 0		1	l v
17+10	2.8538	0.67	1 2	1		l V
17+15	2.8585	0.69	I Q	1	1) V
17+20	2.8633	0.69	IQ	1	1	V
17+25	2.8680	0.69	I Q	1	1	V
17+30	2.8727	0.69	1 2	1	T	v
17+35	2.8775	0.69	1 2	9-	4-	V
17+40	2.8822	0.69	I Q	1	1	V
17+45	2.8870	0.69	Q	Ĵ.	1	V J
17+50	2.8910	0.58	IQ		1	V
17+55	2.8946	0.53	ÎQ	1 a	- î	V
18+ 0	2.8982	0.52	ÎQ	d in	-Ĥ	V
18+ 5	2.9018	0.52	Q	1	- Ť	V
18+10	2.9054	0.52	Q	- C	4	V
18+15	2,9090	0.52	Q		1	v
18+20	2.9126	0.52	Q	2 L	i i	v
18+25	2.9162	0.52	I Q	40		v
18+25	2.9198	0.53	Q		4	v
				2		v
18+35	2.9227	0.42	IQ	5		v V
18+40	2,9251	0.36	10			
18+45	2.9276	0.36	IQ	1	1	V
18+50	2.9293	0.25			10	V
18+55	2.9306	0.19	Q	1	1	V
19+ 0	2.9319	0.18	Q	1.85	1	1 V
19+ 5	2,9339	0.30	10	1	1	V V
19+10	2.9363	0.35	1Q	1		V
19+15	2.9388	0.36	I Q	1	1	I V
19+20	2,9421	0.47	10	1.		V
19+25	2.9457	0.53	Q		1.	I V
19+30	2.9494	0.53	I Q	1		I V
19+35	2.9523	0.43	10	1		I V
19+40	2.9548	0.37	IQ	1	1	1 V
19+45	2.9573	0.36	10	1	1	I V
19+50	2,9591	0.25	10	1	1	I V
19+55	2.9605	0.20	Q	1	1	V V
20+ 0	2.9618	0.19	Q		1	I V
20+ 5	2.9639	0.30	ĨQ	1 î	- i	V V
20+10	2,9664	0.36	IQ	1 i	1	i v
20+15	2.9689	0.37	IQ	1		v v
20+20	2.9715	0.37	IQ	1		v v
20+20	2.9740	0.37		1		v v
		0.37	10			i i
20+30	2.9766		10			1 3
20+35	2.9791	0.37	10			
20+40 20+45	2.9817 2.9842	0.37	10			

20+50	2.9860	0.26	10	1		1	v
20+55	2.9875	0.21	Q	1	- D		V
21+ 0	2,9888	0.20	Q	1		1	V
21+ 5	2.9910	0.31	10	1	.1	1	V
21+10	2.9935	0.37	10	1	1	() () () () () () () () () () () () () (V
21+15	2.9961	0.38	10	1		T	V
21+20	2.9979	0.27	10	1	1	1	V
21+25	2.9994	0.21	Q	1	1		V
21+30	3.0008	0.20	Q	1	1	1	V
21+35	3.0030	0.31	10	1	1	1	V
21+40	3.0055	0.37	10	1	1	1	V
21+45	3.0081	0.38	10	3	1	1	V
21+50	3.0100	0.27	10	1	- Ú.	1	V
21+55	3.0114	0.21	Q	1	1	1	V
22+ 0	3.0129	0.21	Q	4		1	V
22+ 5	3.0151	0.32	10	1		1	V
22+10	3.0176	0.38	10	4	- 11)	V
22+15	3.0203	0.38	10	1	1	1	V
22+20	3.0221	0.27	10	3	1	1	V
22+25	3.0236	0.22	Q	1		1	7
22+30	3.0251	0.21	Q	1	1	1	V
22+35	3.0265	0.21	Q	1	1 () •	1	V
22+40	3.0280	0.21	Q	1	1	1	V
22+45	3.0294	0.21	Q	1	1	Ĩ.	V
22+50	3.0309	0.21	Q	1	1	1	V
22+55	3.0323	0.21	Q	1	1	1	7
23+ 0	3.0338	0.21	Q	1	1	-12-1	Ţ
23+ 5	3.0353	0.21	Q	Ť.	1		V
23+10	3.0367	0.21	Q		-	0.1	V
23+15	3.0382	0.21	Q	1	1		V
23+20	3.0397	0.21	Q	T.	-1		7
23+25	3,0411	0.21	Q	1	-1	0	1
23+30	3.0426	0.21	Q	1	1	10	V
23+35	3.0441	0.21	Q	1	14	1	7
23+40	3.0456	0.21	Q	1	1	1	7
23+45	3.0470	0.21	Q	(Fill	- 1	1	1
23+50	3.0485	0.21	Q	1 I	-1	1	1
23+55	3,0500	0.21	Q	Î	1	1	7
24+ 0	3.0515	0.21	Q	1	- 1C	A	7
24+ 5	3.0520	0.08	Q	1	1	1	Ţ
24+10	3.0520	0.01	Q	1	· · · · · · ·	1	
		ويوالالتحادية		يتوجيكاتديد			

APPENDIX E

REFERENCES

Cover Type (3)	Quality of		Soil	Gro	u
COVER TYPE (5)	Cover (2)	A	В	C	L
NATURAL COVERS -		64			ľ
		h.	1.5	1.0	L
Barren		78	86	91	1
(Rockland, eroded and graded land)					Ľ
Chaparrel, Broadleaf	Poor	53	70	80	1
(Manzonita, ceanothus and scrub oak)	Fair	40	63	75	
 Acceleration of the state of th	Good	31	57	71	
Chaparrel, Narrowleaf	Poor	71	82	88	4
(Chamise and redshank)	Fair	55	72	81	
(Chamise and redshally)	- unit		1~		Ľ
Grass, Annual or Perennial	Poor	67	78	86	
	Fair	50	69	79	Ľ
	Good	38	61	74	
Meadows or Cienegas	Poor	63	77	85	
(Areas with seasonally high water table,	Fair	51	70	80	
principal vegetation is sod forming grass)	Good	30	58	72	1
Description of the second s	Poor	62	76	84	ł
Open Brush (Soft wood shrubs - buckwheat, sage, etc.)	Fair	46	66	77	
(Soft wood Shinds - Sackwheat, sage, etc.)	Good	41	63	75	
	1000	15			
Woodland	Poor Fair	45 36	66 60	77	
(Coniferous or broadleaf trees predominate. Canopy density is at least 50 percent)	Good	28	55	70	
Woodland, Grass	Poor	57	73	82	
(Coniferous or broadleaf trees with canopy	Fair	44	65	77	
density from 20 to 50 percent)	Good	33	58	72	
URBAN COVERS -					
Residential or Commercial Landscaping	Good	32	56	69	
(Lawn, shrubs, etc.)	10112120		1	22.25	
Turf	Poor	58	74	83	
(Irrigated and mowed grass)	Fair	44	65	77	
್ರಾಪ್ಟ್ ಕ್ರಾಮ್ಮನ್ ಸಾಗೂರ್ಯ ಮಾಡುವುದ್ದಿ. ಕ್ರಾಮ್ಮನ್ ಪ್ರೇಮ್ ಕ್ರಾಮ್ಮನ್ ಸ್ಟ್ರಾಮ್ ಕ್ರಾಮ್ಮನ್ ಕ್ರಾಮ್ಮನ್ ಕ್ರಾಮ್ಮನ್ ಕ್ರಾಮ್ಮನ್ ಕ್ರಾಮ್ಮನ್ ಕ್ರಾಮ್ಮನ್ ಕ್ರಾಮ್ಮನ್ ಕ್ರಾಮ	Good	33	58	72	
AGRICULTURAL COVERS -					
Fallow		76	85	90	
(Land plowed but not tilled or seeded)		19	100	1	
		1	<u> </u>	L	1
RCFC & WCD RUNOF	F INDEX	N	UME	BER	s
	FOR				

Cover Type (3)	Quality of	1.000	Soil	Gro	up
cover type (5)	Cover (2)	A	В	C	
AGRICULTURAL COVERS (cont.) -		5	17		
Legumes, Close Seeded	Poor	66	77	85	89
(Alfalfa, sweetclover, timothy, etc.)	Good	58	72	81	85
Orchards, Deciduous		See	Not	e 4	1
(Apples, apricots, pears, walnuts, etc.)	1 A		1	Ĩ.	1
Orchards, Evergreen	Poor	57	73	82	86
(Citrus, avocados, etc.)	Fair	44	65	77	82
	Good	33	58	72	79
Pasture, Dryland	Poor	67	78	86	89
(Annual grasses)	Fair	50	69	79	84
	Good	38	61	74	80
Pasture, Irrigated	Poor	58	74	83	87
(Legumes and perennial grass)	Fair	44	65	77	82
	Good	33	58	72	79
Row Crops	Poor	72	81	88	91
(Field crops - tomatoes, sugar beets, etc.)	Good	67	78	85	89
Small Grain	Poor	65	76	84	88
(Wheat, oats, barley, etc.)	Good	63	75	83	87
Vineyard		See	Not	e 4	

- All runoff index (RI) numbers are for Antecedent Moisture Condition (AMC) II.
- 2. Quality of cover definitions:
 - Poor-Heavily grazed or regularly burned areas. Less than 50 percent of the ground surface is protected by plant cover or brush and tree canopy.
 - Fair-Moderate cover with 50 percent to 75 percent of the ground surface protected.
 - Good-Heavy or dense cover with more than 75 percent of the ground surface protected.
- 3. See Plate C-2 for a detailed description of cover types.
- 4. Use runoff index numbers based on ground cover type. See discussion under "Cover Type Descriptions" on Plate C-2.
- 5. Reference Bibliography item 17.

RCFC & WCD HYDROLOGY MANUAL

RUNOFF INDEX NUMBERS FOR PERVIOUS AREA

PLATE D-5.5 (2 of 2)

Precipitation Frequency Data Server

NOAA Atlas 14, Volume 6, Version 2 Location name: Moreno Valley, California, USA* Latitude: 33.8602°, Longitude: -117.2421° Elevation: 1474.5 ft** * source: ESRI Maps ** source: USGS



POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Sarah Dietz, Sarah Heim, Lillian Hiner, Kazungu Maltaria, Deborah Martin, Sandra Pavlovic, Ishani Roy, Carl Trypaluk, Dale Unruh, Fenglin Yan, Michael Yekta, Tan Zhao, Geoffrey Bonnin, Daniel Brewer, Li-Chuan Chen, Tye Parzybok, John Yarchoan

NOAA, National Weather Service, Silver Spring, Maryland

PF tabular | PF graphical | Maps & aerials

PF tabular

21. 200	1			Avera	ge recurren	ce interval (years)	11 M 11 M 1		
Duration	1	2	5	10	25	50	100	200	500	1000
5-min	0.087 (0,073-0,105)	0.121 (0.101-0.147)	0.167 (0.139-0.203)	0.206 (0.170-0.252)	0.260 (0.207-0.330)	0.303 (0.236-0.393)	0.348 (0.265-0.463)	0.396 (0.292-0.542)	0.464 (0.328-0.663)	0.518 (0.353-0.767
10-min	0.125 (0.104-0.151)	0.174 (0.145-0.210)	0.240 (0.199-0.291)	0.295 (0.243-0.361)	0.373 (0.297-0.472)	0.434 (0.339-0.563)	0.499 (0.379-0.663)	0.568 (0.419-0.777)	0.665 (0.470-0.950)	0.743 (0.506-1.10)
15-min	0.151 (0.126-0.183)	0.210 (0.175-0.254)	0.290 (0.241-0.352)	0.357 (0.294-0.437)	0.451 (0.359-0.571)	0.525 (0.410-0.681)	0.604 (0.459-0.802)	0.687 (0.507-0.940)	0.804 (0.568-1.15)	0.898 (0.612-1.33)
30-min	0.246	0.343	0.473 (0.393-0.574)	0.582	0.735 (0.585-0.931)	0.856 (0.668-1.11)	0.984 (0.748-1.31)	1.12 (0.826-1.53)	1.31 (0.926-1.87)	1.46 (0.998-2.17)
60-min	0.331 (0.277-0.401)	0.461 (0.384-0.558)	0.635 (0.529-0.771)	0.782 (0.645-0.957)	0.988 (0.787-1.25)	1.15 (0.897-1.49)	1.32 (1.00-1.76)	1.51 (1.11-2.06)	1.76 (1.25-2,52)	1.97 (1.34-2,92)
2-hr	0.496 (0.415-0.600)	0.658 (0.550-0.797)	0.875 (0.728-1.06)	1.05 (0.870-1.29)	1.30 (1.04-1.65)	1.50 (1.17-1.94)	1.70 (1.29-2.26)	1.91 (1.41-2.61)	2.20 (1.55-3.14)	2.42 (1.65-3.59)
3-hr	0.613 (0.513-0.742)	0.803 (0.670-0.972)	1.05 (0.877-1.28)	1.26 (1.04-1.54)	1.54 (1.23-1.96)	1.76 (1.38-2.29)	1.99 (1.51-2.65)	2.23 (1.64-3.05)	2.55 (1.80-3.64)	2.80 (1.91-4.15)
6-hr	0.860 (0.718-1.04)	1.12 (0.930-1.35)	1.45 (1.21-1.76)	1.72 (1.42-2.11)	2.09 (1.67-2.66)	2.38 (1.86-3.09)	2.67 (2.03-3.55)	2.97 (2.19-4.07)	3.38 (2.39-4.83)	3.70 (2.52-5.48)
12-hr	1.11 (0.924-1.34)	1.46 (1.22-1.76)	1.91 (1.59-2.32)	2.28 (1.88-2.79)	2.78 (2.22-3.53)	3.17 (2.47-4.10)	3.56 (2.70-4.73)	3.95 (2.92-5.41)	4.49 (3.17-6.42)	4.91 (3.35-7.27)
24-hr	1.40 (1.24-1.61)	1.89 (1.67-2.18)	2.53 (2.23-2.93)	3.05 (2.67-3.56)	3.75 (3.17-4.52)	4.28 (3.55-5.27)	4.82 (3.91-6.07)	5.37 (4.24-6.96)	6.12 (4.63-8.24)	6.69 (4.90-9.32)
2-day	1.61 (1.43-1.86)	2.22 (1.96-2.56)	3.01 (2.65-3.48)	3.65 (3.19-4.26)	4.52 (3.83-5.45)	5.19 (4.31-6.38)	5.87 (4.75-7.39)	6.56 (5.17-8.49)	7.50 (5.68-10.1)	8.23 (6.03-11.5)
3-day	1.71 (1.51-1.97)	2.38 (2.10-2.75)	3.25 (2.87-3.77)	3.97 (3.47-4.63)	4.94 (4.18-5.96)	5.69 (4.72-7.00)	6.45 (5.23-8.13)	7.24 (5.71-9.37)	8.31 (6.29-11.2)	9.15 (6.70-12.7)
4-day	1.85 (1.63-2.13)	2.58 (2.28-2.98)	3.56 (3.14-4.12)	4.36 (3.81-5.09)	5.45 (4.62-6,57)	6.30 (5.22-7.75)	7.16 (5.80-9.02)	8.05 (6.35-10.4)	9.27 (7.02-12.5)	10.2 (7.48-14.2)
7-day	1.97 (1.74-2.27)	2.81 (2.48-3.24)	3.93 (3.46-4.55)	4.85 (4.24-5.66)	6.12 (5.18-7.37)	7.10 (5.89-8.73)	8.11 (6.57-10.2)	9.16 (7.22-11.9)	10.6 (8.02-14.3)	11.7 (8.58-16.3)
10-day	1.99 (1.76-2.29)	2.87 (2.53-3.31)	4.05 (3.57-4.69)	5.03 (4.40-5.87)	6.39 (5.41-7.70)	7.45 (6.18-9.16)	8.54 (6.92-10.8)	9.68 (7.63-12.5)	11.3 (8.52-15.2)	12.5 (9.14-17.4)
20-day	2.21 (1.96-2.55)	3.24 (2.87-3.74)	4.67 (4.11-5.40)	5.87 (5.13-6.85)	7.57 (6.41-9.13)	8.93 (7.41-11.0)	10.4 (8.39-13.0)	11.9 (9.35-15.3)	13.9 (10.6-18.8)	15.6 (11.4-21.7)
30-day	2.44 (2.16-2.82)	3.59 (3.17-4.14)	5.20 (4.58-6.02)	6.59 (5.76-7.69)	8.59 (7.27-10.3)	10.2 (8.46-12.5)	11.9 (9.64-15.0)	13.7 (10.8-17.8)	16.3 (12.3-21.9)	18.3 (13.4-25.5)
45-day	2.81 (2.48-3.24)	4.08 (3.61-4.71)	5.91 (5.21-6.84)	7.51 (6.57-8.77)	9.88 (8.36-11.9)	11.8 (9.81-14.5)	13.9 (11.3-17.5)	16.2 (12.7-20.9)	19.4 (14.7-26.1)	22.0 (16.1-30.6)
60-day	3.13 (2.77-3.61)	4.48 (3.96-5.17)	6.46 (5.70-7.49)	8.23 (7.20-9.61)	10.9 (9.20-13.1)	13.1 (10.9-16.1)	15.5 (12.5-19.5)	18.1 (14.3-23.4)	21.9 (16.6-29.5)	25.0 (18.3-34.8)

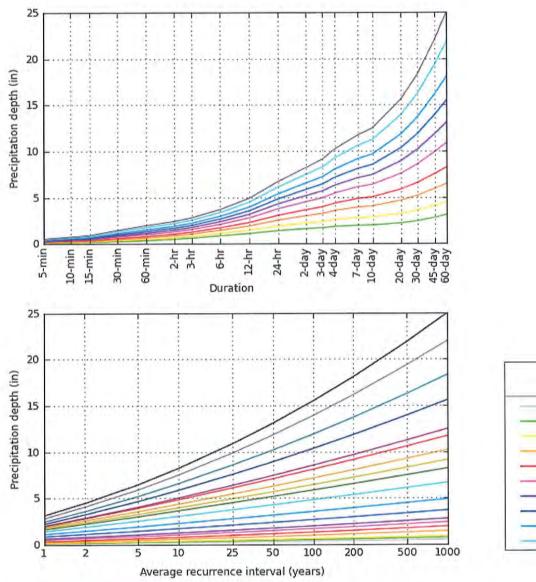
¹ Pracipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

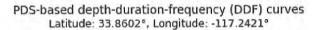
Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values.

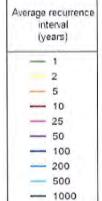
Please refer to NOAA Atlas 14 document for more information.

Back to Top

PF graphical







Dura	ation
— 5-min	— 2-day
— 10-min	- 3-day
- 15-min	— 4-day
- 30-min	— 7-day
- 60-min	- 10-day
- 2-hr	— 20-day
- 3-hr	— 30-day
— 6-hr	- 45-day
- 12-hr	— 60-day
- 24-hr	

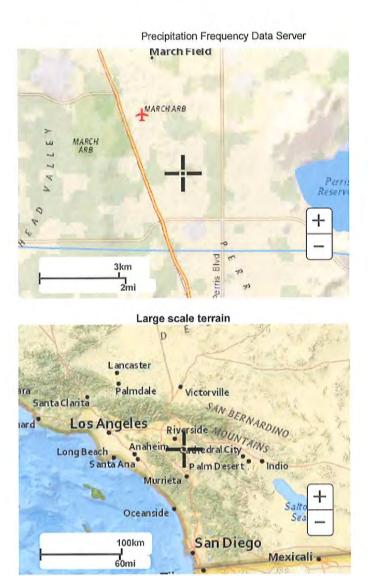
NOAA Atlas 14, Volume 6, Version 2

Created (GMT): Wed May 12 14:24:52 2021

Back to Top

Maps & aerials

Small scale terrain





Large scale aerial

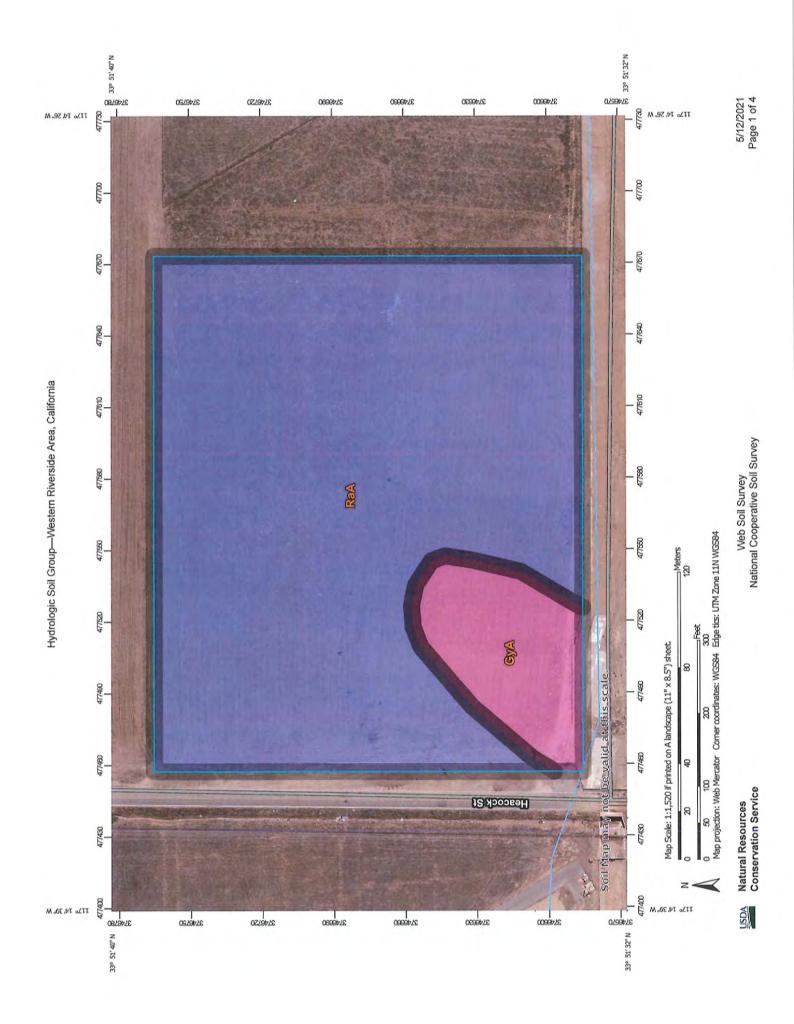
Precipitation Frequency Data Server



Back to Top

US Department of Commerce National Oceanic and Atmospheric Administration National Weather Service National Water Center 1325 East West Highway Silver Spring, MD 20910 Questions?: HDSC.Questions@noaa.gov

Disclaimer



Hydrologic Soil Group-Western Riverside Area, California

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Area of Interest (AOI) CD Area of Interest (AOI) Not rated or not available AD Natar Features BD Streams and Canals BD Streams and Canals CD Us Routes D Interstate Highways CD Us Routes D Nation Reads CD Us Routes D Not rated or not available AD Us Routes D Not rated or not available AD Not rated or not available AD Not rated or not available AD Anajor Roads AD Analy Notography AD<	Area of Interest (AOI)			The soil surveys that comprise your AOI were mapped at
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or not available Local Roads Background Aerial Photography or not available	•	2	vajor Roads	Web Soil Survey URL: Coordinate System: Web Mercator (EPSG:3857)
Background Aerial Photography or not available	Not rated or not available		ocal Roads	Maps from the Web Soil Survey are based on the Web Merc
or not available	Soil Rating Lines	Backmond		projection, which preserves direction and shape but distorts
or not available	A A	A Manual A	Verial Photography	distance and area. A projection that preserves area, such as Albers equal-area contribution should be used if more
or not available				accurate calculations of distance or area are required.
or not available	8			This product is generated from the USDA-NRCS certified data as
or not available	BID -			of the version date(s) listed below.
or not available	0			Soil Survey Area: Western Riverside Area, California Survey Area Data: Version 13. May 27, 2020
or not available	C/D			Coll more units and Inholod (on oncore official for more station
or not available	0			Soli map units are lapered (as space allows) for map scares 1:50,000 or larger.
				Date(s) aerial images were photographed: May 25, 2019-Jun
A B D	Soil Rating Points			25, 2019
BU AD	A D			The orthophoto or other base map on which the soil lines we
B/D	12			complied and digitized propably directs from the packground imagery displayed on these maps. As a result, some minor
	8			shifting of map unit boundaries may be evident.

5/12/2021 Page 2 of 4

> USDA Natural Resources Conservation Service

Web Soil Survey National Cooperative Soil Survey

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
GyA	Greenfield sandy loam, 0 to 2 percent slopes	A	1.1	11.4%
RaA	Ramona sandy loam, 0 to 2 percent slopes, MLRA 19	В	8.5	88.6%
Totals for Area of Inter	rest		9.6	100.0%

Hydrologic Soil Group

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

Rating Options

Aggregation Method: Dominant Condition Component Percent Cutoff: None Specified Tie-break Rule: Higher

PROJE	CT INFORMATION
ENGINEERED PRODUCT MANAGER	
ADS SALES REP	
PROJECT NO.	



1482-0001 HEACOCK PARKING LOT MORENO VALLEY, CA

MC-4500 STORMTECH CHAMBER SPECIFICATIONS

- CHAMBERS SHALL BE STORMTECH MC-4500. 1.
- CHAMBERS SHALL BE ARCH-SHAPED AND SHALL BE MANUFACTURED FROM VIRGIN, IMPACT-MODIFIED POLYPROPYLENE 2. COPOLYMERS.
- CHAMBERS SHALL MEET THE REQUIREMENTS OF ASTM F2418-16a, "STANDARD SPECIFICATION FOR POLYPROPYLENE (PP) 3. CORRUGATED WALL STORMWATER COLLECTION CHAMBERS" CHAMBER CLASSIFICATION 60x101.
- CHAMBER ROWS SHALL PROVIDE CONTINUOUS, UNOBSTRUCTED INTERNAL SPACE WITH NO INTERNAL SUPPORTS THAT WOULD 4. IMPEDE FLOW OR LIMIT ACCESS FOR INSPECTION.
- THE STRUCTURAL DESIGN OF THE CHAMBERS, THE STRUCTURAL BACKFILL, AND THE INSTALLATION REQUIREMENTS SHALL ENSURE 5. THAT THE LOAD FACTORS SPECIFIED IN THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, SECTION 12.12, ARE MET FOR: 1) LONG-DURATION DEAD LOADS AND 2) SHORT-DURATION LIVE LOADS, BASED ON THE AASHTO DESIGN TRUCK WITH CONSIDERATION FOR IMPACT AND MULTIPLE VEHICLE PRESENCES.
- CHAMBERS SHALL BE DESIGNED, TESTED AND ALLOWABLE LOAD CONFIGURATIONS DETERMINED IN ACCORDANCE WITH ASTM F2787, 6 "STANDARD PRACTICE FOR STRUCTURAL DESIGN OF THERMOPLASTIC CORRUGATED WALL STORMWATER COLLECTION CHAMBERS". LOAD CONFIGURATIONS SHALL INCLUDE: 1) INSTANTANEOUS (<1 MIN) AASHTO DESIGN TRUCK LIVE LOAD ON MINIMUM COVER 2) MAXIMUM PERMANENT (75-YR) COVER LOAD AND 3) ALLOWABLE COVER WITH PARKED (1-WEEK) AASHTO DESIGN TRUCK.

7 REQUIREMENTS FOR HANDLING AND INSTALLATION:

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- TO MAINTAIN THE WIDTH OF CHAMBERS DURING SHIPPING AND HANDLING, CHAMBERS SHALL HAVE INTEGRAL, INTERLOCKING STACKING LUGS
- TO ENSURE A SECURE JOINT DURING INSTALLATION AND BACKFILL, THE HEIGHT OF THE CHAMBER JOINT SHALL NOT BE LESS THAN 3"
- TO ENSURE THE INTEGRITY OF THE ARCH SHAPE DURING INSTALLATION, a) THE ARCH STIFFNESS CONSTANT AS DEFINED IN SECTION 6.2.8 OF ASTM F2418 SHALL BE GREATER THAN OR EQUAL TO 500 LBS/IN/IN. AND b) TO RESIST CHAMBER DEFORMATION DURING INSTALLATION AT ELEVATED TEMPERATURES (ABOVE 73° F / 23° C), CHAMBERS SHALL BE PRODUCED FROM REFLECTIVE GOLD OR YELLOW COLORS.
- ONLY CHAMBERS THAT ARE APPROVED BY THE SITE DESIGN ENGINEER WILL BE ALLOWED. UPON REQUEST BY THE SITE DESIGN 8. ENGINEER OR OWNER, THE CHAMBER MANUFACTURER SHALL SUBMIT A STRUCTURAL EVALUATION FOR APPROVAL BEFORE DELIVERING CHAMBERS TO THE PROJECT SITE AS FOLLOWS:
 - THE STRUCTURAL EVALUATION SHALL BE SEALED BY A REGISTERED PROFESSIONAL ENGINEER.
 - THE STRUCTURAL EVALUATION SHALL DEMONSTRATE THAT THE SAFETY FACTORS ARE GREATER THAN OR EQUAL TO 1.95 FOR DEAD LOAD AND 1.75 FOR LIVE LOAD, THE MINIMUM REQUIRED BY ASTM F2787 AND BY SECTIONS 3 AND 12.12 OF THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS FOR THERMOPLASTIC PIPE.
 - THE TEST DERIVED CREEP MODULUS AS SPECIFIED IN ASTM F2418 SHALL BE USED FOR PERMANENT DEAD LOAD DESIGN EXCEPT THAT IT SHALL BE THE 75-YEAR MODULUS USED FOR DESIGN.
- CHAMBERS AND END CAPS SHALL BE PRODUCED AT AN ISO 9001 CERTIFIED MANUFACTURING FACILITY.

IMPORTANT - NOTES FOR THE BIDDING AND INSTALLATION OF MC-4500 CHAMBER SYSTEM

- 1 STORMTECH MC-4500 CHAMBERS SHALL NOT BE INSTALLED UNTIL THE MANUFACTURER'S REPRESENTATIVE HAS COMPLETED A PRE-CONSTRUCTION MEETING WITH THE INSTALLERS.
- 2 STORMTECH MC-4500 CHAMBERS SHALL BE INSTALLED IN ACCORDANCE WITH THE "STORMTECH MC-3500/MC-4500 CONSTRUCTION GUIDE".
- CHAMBERS ARE NOT TO BE BACKFILLED WITH A DOZER OR EXCAVATOR SITUATED OVER THE CHAMBERS. 3 STORMTECH RECOMMENDS 3 BACKFILL METHODS:
 - STONESHOOTER LOCATED OFF THE CHAMBER BED.
 - BACKFILL AS ROWS ARE BUILT USING AN EXCAVATOR ON THE FOUNDATION STONE OR SUBGRADE.
 - BACKFILL FROM OUTSIDE THE EXCAVATION USING A LONG BOOM HOE OR EXCAVATOR.
- THE FOUNDATION STONE SHALL BE LEVELED AND COMPACTED PRIOR TO PLACING CHAMBERS. 4
- 5. JOINTS BETWEEN CHAMBERS SHALL BE PROPERLY SEATED PRIOR TO PLACING STONE.
- 6. MAINTAIN MINIMUM 9" (230 mm) SPACING BETWEEN THE CHAMBER ROWS.
- INLET AND OUTLET MANIFOLDS MUST BE INSERTED A MINIMUM OF 12" (300 mm) INTO CHAMBER END CAPS. 7.
- 8. EMBEDMENT STONE SURROUNDING CHAMBERS MUST BE A CLEAN, CRUSHED, ANGULAR STONE MEETING THE AASHTO M43 DESIGNATION OF #3 OR #4
- STONE SHALL BE BROUGHT UP EVENLY AROUND CHAMBERS SO AS NOT TO DISTORT THE CHAMBER SHAPE. STONE DEPTHS SHOULD NEVER 9. DIFFER BY MORE THAN 12" (300 mm) BETWEEN ADJACENT CHAMBER ROWS.
- 10. STONE MUST BE PLACED ON THE TOP CENTER OF THE CHAMBER TO ANCHOR THE CHAMBERS IN PLACE AND PRESERVE ROW SPACING.
- 11. THE CONTRACTOR MUST REPORT ANY DISCREPANCIES WITH CHAMBER FOUNDATION MATERIAL BEARING CAPACITIES TO THE SITE DESIGN. ENGINEER
- 12. ADS RECOMMENDS THE USE OF "FLEXSTORM CATCH IT" INSERTS DURING CONSTRUCTION FOR ALL INLETS TO PROTECT THE SUBSURFACE STORMWATER MANAGEMENT SYSTEM FROM CONSTRUCTION SITE RUNOFF.

NOTES FOR CONSTRUCTION EQUIPMENT

- 1. STORMTECH MC-4500 CHAMBERS SHALL BE INSTALLED IN ACCORDANCE WITH THE "STORMTECH MC-3500/MC-4500 CONSTRUCTION GUIDE".
- 2. THE USE OF EQUIPMENT OVER MC-4500 CHAMBERS IS LIMITED:
 - NO EQUIPMENT IS ALLOWED ON BARE CHAMBERS.
 - WITH THE "STORMTECH MC-3500/MC-4500 CONSTRUCTION GUIDE"
 - WEIGHT LIMITS FOR CONSTRUCTION EQUIPMENT CAN BE FOUND IN THE "STORMTECH MC-3500/MC-4500 CONSTRUCTION GUIDE".
- FULL 36" (900 mm) OF STABILIZED COVER MATERIALS OVER THE CHAMBERS IS REQUIRED FOR DUMP TRUCK TRAVEL OR DUMPING.

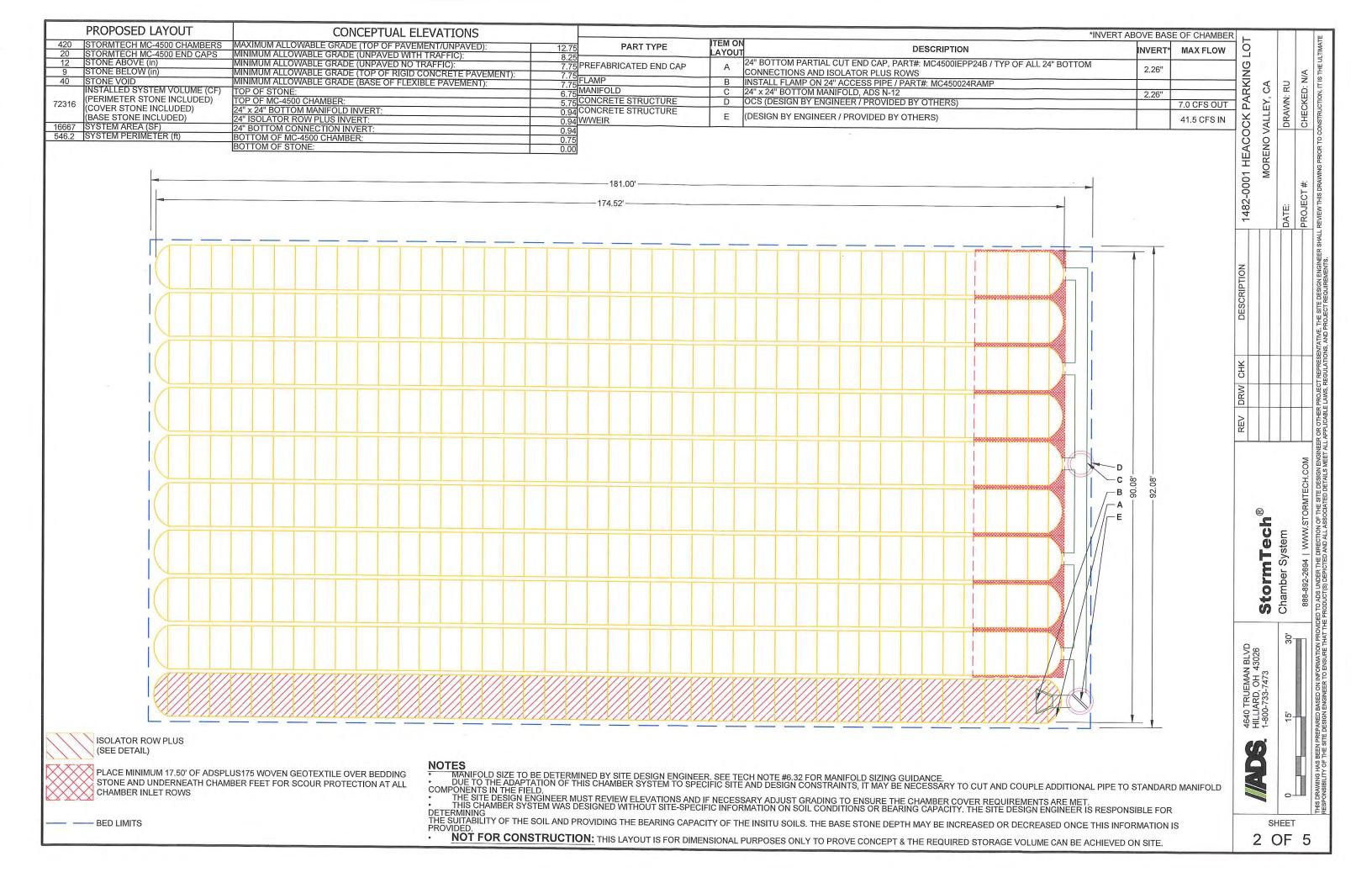
USE OF A DOZER TO PUSH EMBEDMENT STONE BETWEEN THE ROWS OF CHAMBERS MAY CAUSE DAMAGE TO CHAMBERS AND IS NOT AN ACCEPTABLE BACKFILL METHOD. ANY CHAMBERS DAMAGED BY USING THE "DUMP AND PUSH" METHOD ARE NOT COVERED UNDER THE STORMTECH STANDARD WARRANTY.

CONTACT STORMTECH AT 1-888-892-2694 WITH ANY QUESTIONS ON INSTALLATION REQUIREMENTS OR WEIGHT LIMITS FOR CONSTRUCTION EQUIPMENT.



SiteASSIST FOR STORMTECH INSTRUCTIONS, DOWNLOAD THE INSTALLATION APP

NO RUBBER TIRED LOADER, DUMP TRUCK, OR EXCAVATORS ARE ALLOWED UNTIL PROPER FILL DEPTHS ARE REACHED IN ACCORDANCE



ACCEPTABLE FILL MATERIALS: STORMTECH MC-4500 CHAMBER SYSTEMS

	MATERIAL LOCATION	DESCRIPTION	AASHTO MATERIAL CLASSIFICATIONS	COMPACTION / DENSITY REQUIREMENT
D	FINAL FILL: FILL MATERIAL FOR LAYER 'D' STARTS FROM THE TOP OF THE 'C' LAYER TO THE BOTTOM OF FLEXIBLE PAVEMENT OR UNPAVED FINISHED GRADE ABOVE. NOTE THAT PAVEMENT SUBBASE MAY BE PART OF THE 'D' LAYER	ANY SOIL/ROCK MATERIALS, NATIVE SOILS, OR PER ENGINEER'S PLANS. CHECK PLANS FOR PAVEMENT SUBGRADE REQUIREMENTS.	N/A	PREPARE PER SITE DESIGN ENGINEER'S PLANS. PAVED INSTALLATIONS MAY HAVE STRINGENT MATERIAL AND PREPARATION REQUIREMENTS.
С	INITIAL FILL: FILL MATERIAL FOR LAYER 'C' STARTS FROM THE TOP OF THE EMBEDMENT STONE ('B' LAYER) TO 24" (600 mm) ABOVE THE TOP OF THE CHAMBER. NOTE THAT PAVEMENT SUBBASE MAY BE A PART OF THE 'C' LAYER.	GRANULAR WELL-GRADED SOIL/AGGREGATE MIXTURES, <35% FINES OR PROCESSED AGGREGATE. MOST PAVEMENT SUBBASE MATERIALS CAN BE USED IN LIEU OF THIS LAYER.	AASHTO M145 ¹ A-1, A-2-4, A-3 OR AASHTO M43 ¹ 3, 357, 4, 467, 5, 56, 57, 6, 67, 68, 7, 78, 8, 89, 9, 10	BEGIN COMPACTIONS AFTER 24" (600 mm) OF MATERIAL OVER THE CHAMBERS IS REACHED. COMPACT ADDITIONAL LAYERS IN 12" (300 mm) MAX LIFTS TO A MIN. 95% PROCTOR DENSITY FOR WELL GRADED MATERIAL AND 95% RELATIVE DENSITY FOR PROCESSED AGGREGATE MATERIALS.
В	EMBEDMENT STONE: FILL SURROUNDING THE CHAMBERS FROM THE FOUNDATION STONE ('A' LAYER) TO THE 'C' LAYER ABOVE.	CLEAN, CRUSHED, ANGULAR STONE	AASHTO M43 ¹ 3, 4	NO COMPACTION REQUIRED.
A	FOUNDATION STONE: FILL BELOW CHAMBERS FROM THE SUBGRADE UP TO THE FOOT (BOTTOM) OF THE CHAMBER.	CLEAN, CRUSHED, ANGULAR STONE	AASHTO M43 ¹ 3, 4	PLATE COMPACT OR ROLL TO ACHIEVE A FLAT SURFACE. ^{2,3}

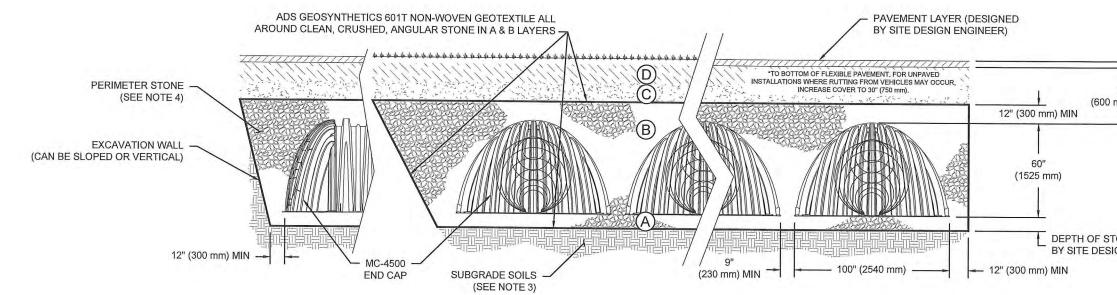
PLEASE NOTE:

THE LISTED AASHTO DESIGNATIONS ARE FOR GRADATIONS ONLY. THE STONE MUST ALSO BE CLEAN, CRUSHED, ANGULAR. FOR EXAMPLE, A SPECIFICATION FOR #4 STONE WOULD STATE: "CLEAN, CRUSHED, ANGULAR NO. 4 (AASHTO M43) STONE".

STORMTECH COMPACTION REQUIREMENTS ARE MET FOR 'A' LOCATION MATERIALS WHEN PLACED AND COMPACTED IN 9" (230 mm) (MAX) LIFTS USING TWO FULL COVERAGES WITH A VIBRATORY COMPACTOR.

WHERE INFILTRATION SURFACES MAY BE COMPROMISED BY COMPACTION, FOR STANDARD DESIGN LOAD CONDITIONS, A FLAT SURFACE MAY BE ACHIEVED BY RAKING OR DRAGGING WITHOUT COMPACTION EQUIPMENT. FOR SPECIAL LOAD DESIGNS, CONTACT STORMTECH FOR 3 COMPACTION REQUIREMENTS. 4

ONCE LAYER 'C' IS PLACED, ANY SOIL/MATERIAL CAN BE PLACED IN LAYER 'D' UP TO THE FINISHED GRADE. MOST PAVEMENT SUBBASE SOILS CAN BE USED TO REPLACE THE MATERIAL REQUIREMENTS OF LAYER 'C' OR 'D' AT THE SITE DESIGN ENGINEER'S DISCRETION.



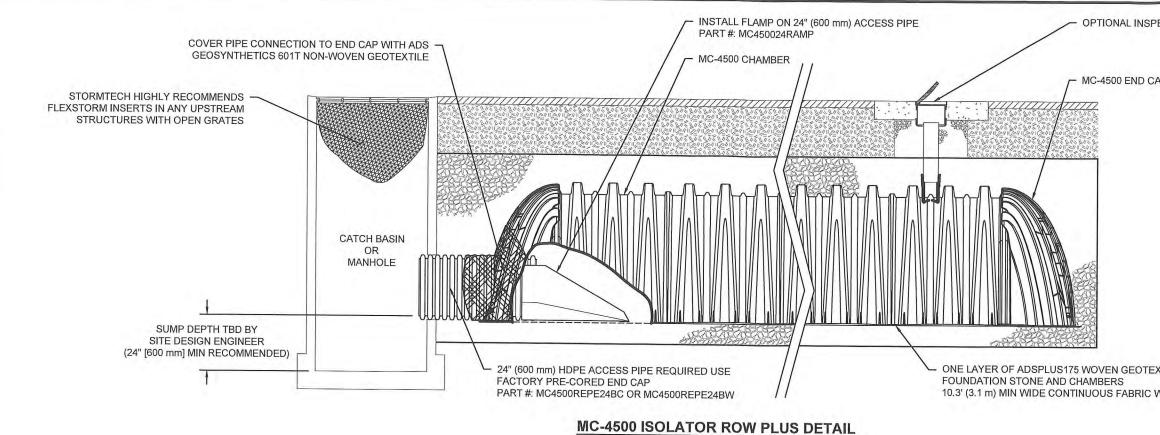
NOTES:

- CHAMBERS SHALL MEET THE REQUIREMENTS OF ASTM F2418-16a, "STANDARD SPECIFICATION FOR POLYPROPYLENE (PP) CORRUGATED WALL STORMWATER COLLECTION CHAMBERS" CHAMBER CLASSIFICATION 60x101 1.
- MC-4500 CHAMBERS SHALL BE DESIGNED IN ACCORDANCE WITH ASTM F2787 "STANDARD PRACTICE FOR STRUCTURAL DESIGN OF THERMOPLASTIC CORRUGATED WALL STORMWATER COLLECTION CHAMBERS". 2.
- 3. THE SITE DESIGN ENGINEER IS RESPONSIBLE FOR ASSESSING THE BEARING RESISTANCE (ALLOWABLE BEARING CAPACITY) OF THE SUBGRADE SOILS AND THE DEPTH OF FOUNDATION STONE WITH CONSIDERATION
- FOR THE RANGE OF EXPECTED SOIL MOISTURE CONDITIONS.
- 4. PERIMETER STONE MUST BE EXTENDED HORIZONTALLY TO THE EXCAVATION WALL FOR BOTH VERTICAL AND SLOPED EXCAVATION WALLS.
- REQUIREMENTS FOR HANDLING AND INSTALLATION: 5
 - TO MAINTAIN THE WIDTH OF CHAMBERS DURING SHIPPING AND HANDLING, CHAMBERS SHALL HAVE INTEGRAL, INTERLOCKING STACKING LUGS.
 - TO ENSURE A SECURE JOINT DURING INSTALLATION AND BACKFILL, THE HEIGHT OF THE CHAMBER JOINT SHALL NOT BE LESS THAN 3".
 - TO ENSURE THE INTEGRITY OF THE ARCH SHAPE DURING INSTALLATION, a) THE ARCH STIFFNESS CONSTANT AS DEFINED IN SECTION 6.2.8 OF ASTM F2418 SHALL BE GREATER THAN OR EQUAL TO 500 LBS/IN/IN. AND b) TO RESIST CHAMBER DEFORMATION DURING INSTALLATION AT ELEVATED TEMPERATURES (ABOVE 73° F / 23° C), CHAMBERS SHALL BE PRODUCED FROM REFLECTIVE GOLD OR YELLOW COLORS.

T	+
t 24'' nm) MIN*	7.0' (2.1 m) MAX
1	1

DEPTH OF STONE TO BE DETERMINED BY SITE DESIGN ENGINEER 9" (230 mm) MIN

-		4640 TRUEMAN BLVD HILLARD OH 43026	(REV	REV DRW CHK	CHK	DESCRIPTION	1482-0001 HEAC	482-0001 HEACOCK PARKING LOT
SI		1-800-733-7473	StormTech					MORENO	MORENO VALLEY, CA
			Chamber System					DATE:	DRAWN: RU
5			888-892-2694 WWW.STORMTECH.COM					PROJECT #:	CHECKED: N/A
	THIS DRAWING HAS BEEN PI RESPONSIBILITY OF THE SIT	REPARED BASED ON INFORMATION PRO	THIS DRAWING HAS BEEN PREPARED BASED ON INFORMATION PROVIDED TO ADS UNDER THE DIRECTION OF THE SITE DESIGN ENGINEER OR OTHER PROJECT REPRESENTATIVE. THE SITE DESIGN ENGINEER AND ALL ASSOCIATED DETAILS THE ULTIMATE RESOLUTIONS, AND PROJECT REQUIREMENTS.	EER OR OTHEF	REAUS, RE	REPRESENT/ GULATIONS,	ATIVE. THE SITE DESIGN ENGINEER SH AND PROJECT REQUIREMENTS.	ALL REVIEW THIS DRAWING PRIOR TO	CONSTRUCTION. IT IS THE ULTIMA



NTS

INSPECTION & MAINTENANCE

STEP 1) INSPECT ISOLATOR ROW PLUS FOR SEDIMENT

- A. INSPECTION PORTS (IF PRESENT)
- A.1. REMOVE/OPEN LID ON NYLOPLAST INLINE DRAIN
- A.2. REMOVE AND CLEAN FLEXSTORM FILTER IF INSTALLED
- A.3. USING A FLASHLIGHT AND STADIA ROD, MEASURE DEPTH OF SEDIMENT AND RECORD ON MAINTENANCE LOG
- A.4. LOWER A CAMERA INTO ISOLATOR ROW PLUS FOR VISUAL INSPECTION OF SEDIMENT LEVELS (OPTIONAL)
- A.5. IF SEDIMENT IS AT, OR ABOVE, 3" (80 mm) PROCEED TO STEP 2. IF NOT, PROCEED TO STEP 3.
- B. ALL ISOLATOR PLUS ROWS
- B.1. REMOVE COVER FROM STRUCTURE AT UPSTREAM END OF ISOLATOR ROW PLUS
- B.2. USING A FLASHLIGHT, INSPECT DOWN THE ISOLATOR ROW PLUS THROUGH OUTLET PIPE
 - i) MIRRORS ON POLES OR CAMERAS MAY BE USED TO AVOID A CONFINED SPACE ENTRY
 - ii) FOLLOW OSHA REGULATIONS FOR CONFINED SPACE ENTRY IF ENTERING MANHOLE
- B.3. IF SEDIMENT IS AT, OR ABOVE, 3" (80 mm) PROCEED TO STEP 2. IF NOT, PROCEED TO STEP 3.
- STEP 2) CLEAN OUT ISOLATOR ROW PLUS USING THE JETVAC PROCESS
 - A. A FIXED CULVERT CLEANING NOZZLE WITH REAR FACING SPREAD OF 45" (1.1 m) OR MORE IS PREFERRED
 - B. APPLY MULTIPLE PASSES OF JETVAC UNTIL BACKFLUSH WATER IS CLEAN
 - C. VACUUM STRUCTURE SUMP AS REQUIRED
- STEP 3) REPLACE ALL COVERS, GRATES, FILTERS, AND LIDS; RECORD OBSERVATIONS AND ACTIONS.
- STEP 4) INSPECT AND CLEAN BASINS AND MANHOLES UPSTREAM OF THE STORMTECH SYSTEM.

NOTES

- 1. INSPECT EVERY 6 MONTHS DURING THE FIRST YEAR OF OPERATION. ADJUST THE INSPECTION INTERVAL BASED ON PREVIOUS OBSERVATIONS OF SEDIMENT ACCUMULATION AND HIGH WATER ELEVATIONS.
- 2. CONDUCT JETTING AND VACTORING ANNUALLY OR WHEN INSPECTION SHOWS THAT MAINTENANCE IS NECESSARY.

TION PORT	CK PARKING LOT	ALLEY, CA	DRAWN: RU	CHECKED: N/A
	1482-0001 HEACOCK PARKING LOT	MORENO VALLEY, CA	DATE:	PROJECT #. 0
	DESCRIPTION			
BETWEEN DUT SEAMS	REV DRW CHK			
		StormTech®	Chamber System	888-892-2694 WWW.STORMTECH.COM
		1-800-733-7473		
		SI		5

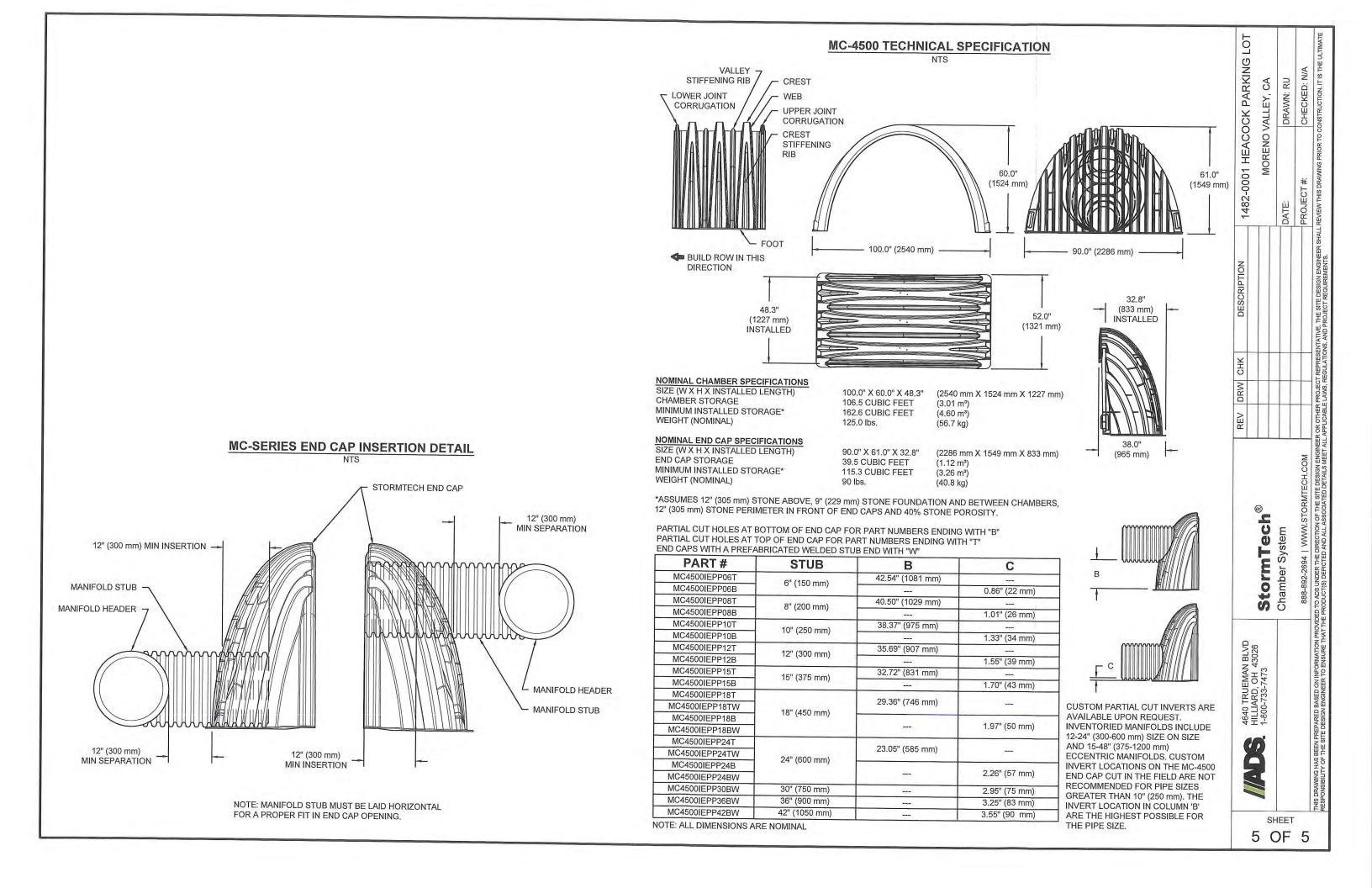


EXHIBIT A

HYDROLOGY MAP (EXISTING CONDITION; RATIONAL METHOD)

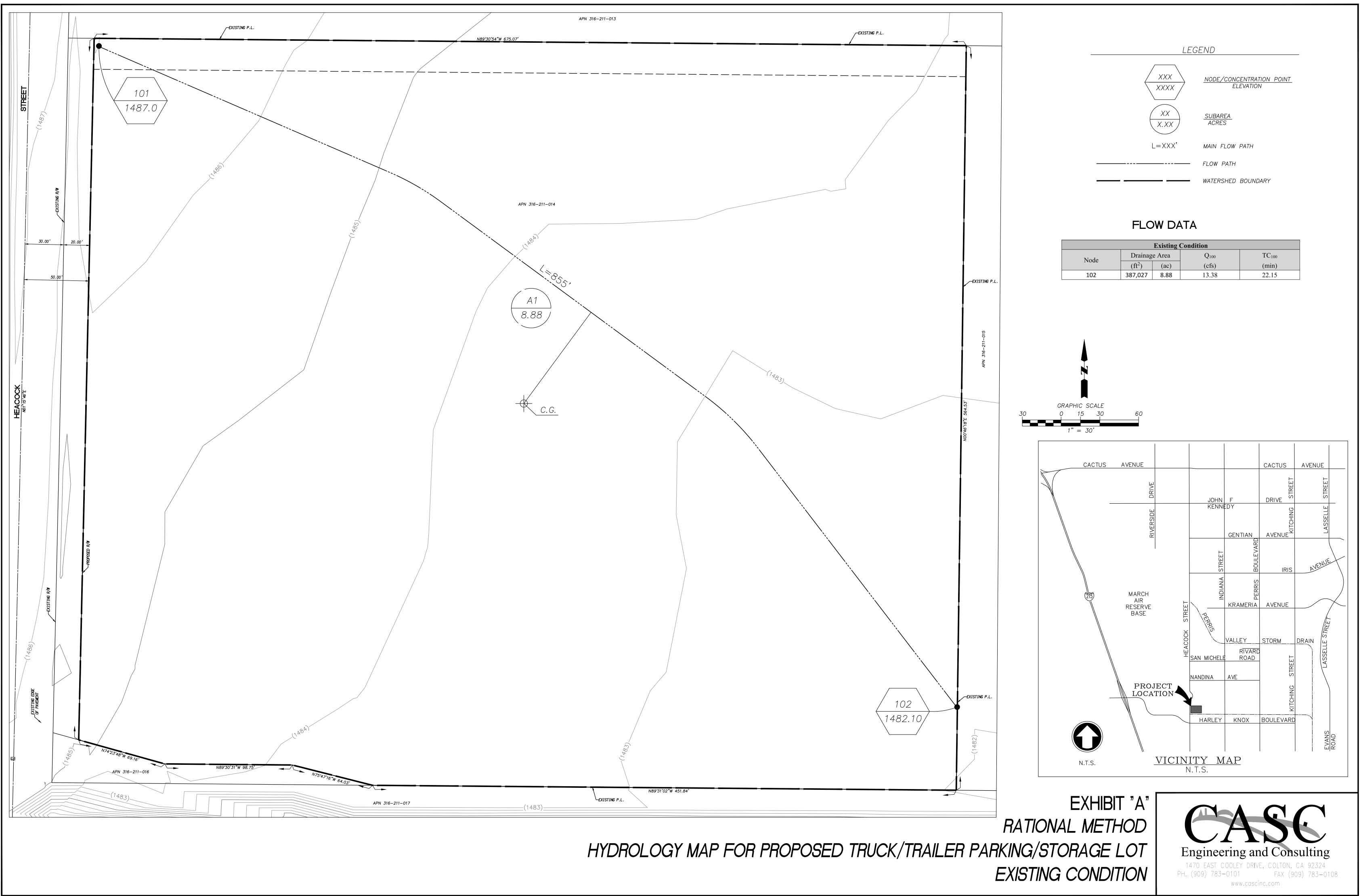


EXHIBIT B

HYDROLOGY MAP (PROPOSED CONDITION; RATIONAL METHOD)

